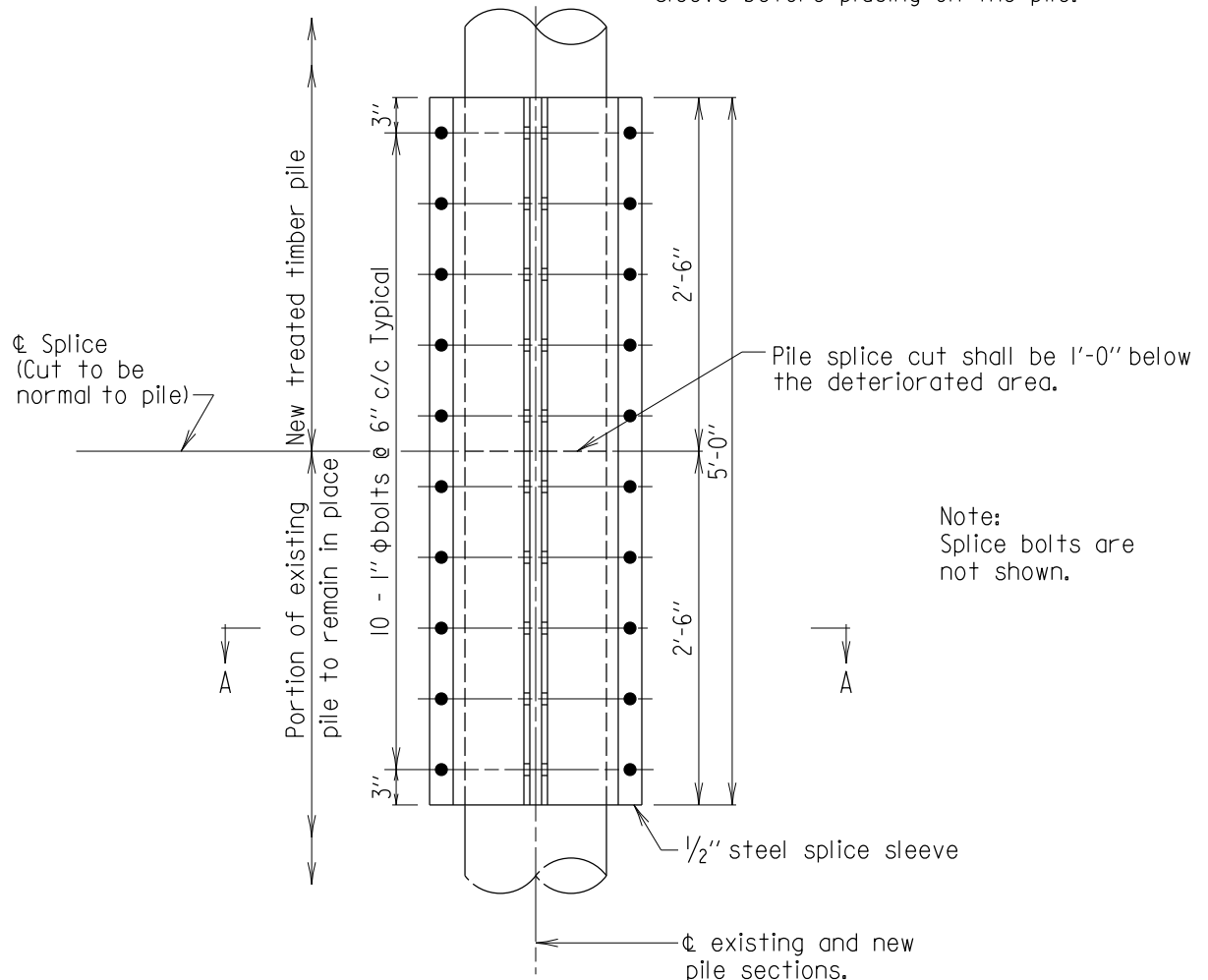


**Note:**

Butt ends of new treated timber pile and existing pile shall have the same diameter. All voids between the existing pile and the new pile section or the pile and the sleeve shall be filled with an epoxy. The epoxy shall be placed on the existing pile end before installing new section and on inside of the sleeve before placing on the pile.



**Note:**  
Splice bolts are not shown.

**ELEVATION**  
**PILE SPLICE SLEEVE**  
Scale:  $\frac{3}{4}'' = 1'-0''$

**Notes:**

1. Epoxy shall be water insensitive with a consistency of putty.
2. All structural steel shall be ASTM A 709 Grade 36 and be hot-dipped galvanized after fabrication in conformance with ASTM A 153.
3. Hardware shall be ASTM A 325 and be mechanically galvanized in conformance with ASTM A 153.
4. All timber for cross bracing and piling shall conform to Section 462. All timber for new cross bracing shall be No. 1 Southern Pine. All timber for piles shall be Southern Pine. All timber shall be treated with creosote with 20 lb/ft<sup>3</sup> retention in conformance with AASHTO M 133.
5. For "Section A-A" see sheet nos. 2 and 3 of 8.

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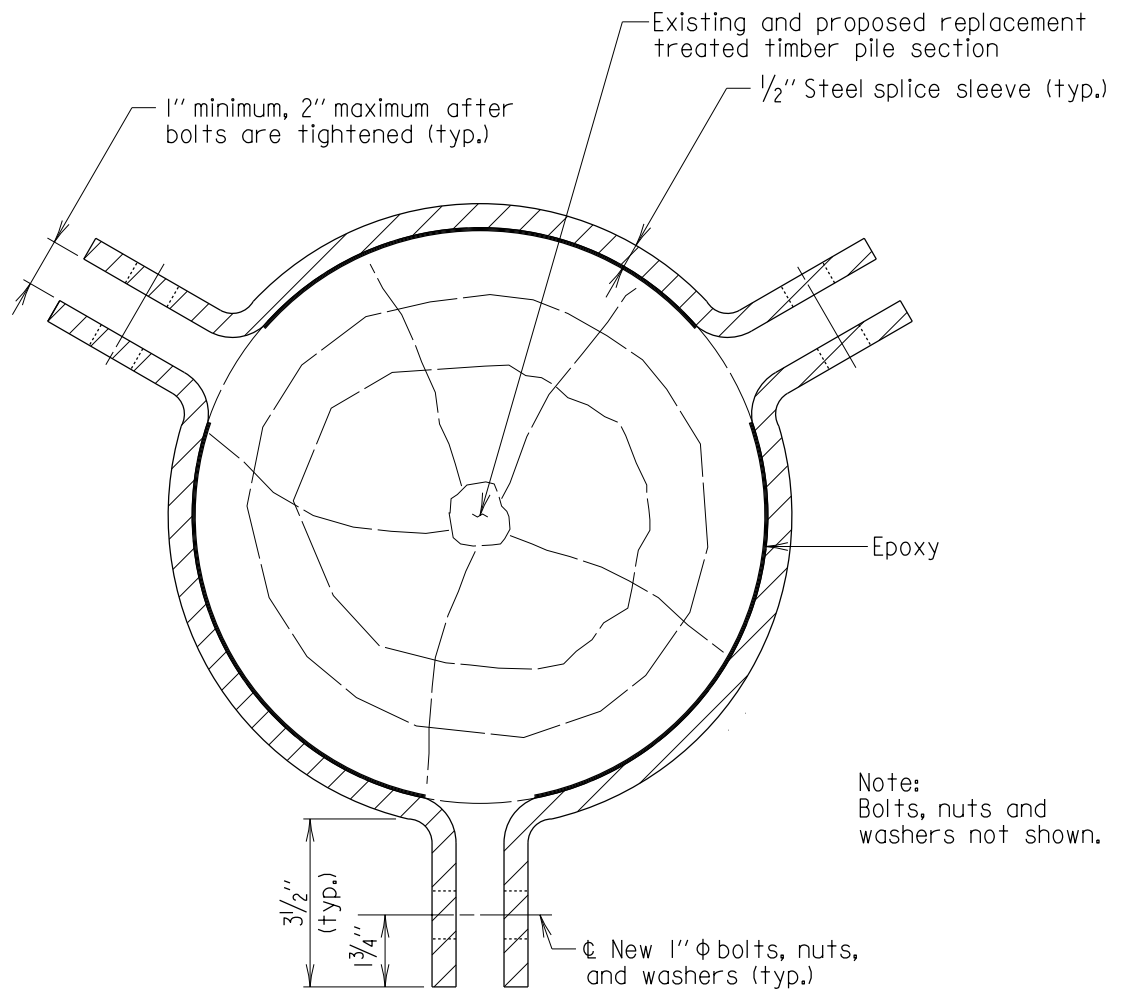
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STATE HIGHWAY ADMINISTRATION  
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**SPLICE FOR CONNECTING EXISTING TIMBER  
PILING TO NEW TIMBER PILING**

STANDARD NO. BR-SR(0.01)-95-304

SHEET 1 OF 8

STRUCTURAL REPAIRS



SECTION A-A (3 SECTION SPLICE ALTERNATE)

Scale: 3" = 1'-0"

Note:  
The three section splice can only be used when there is no bracing being attached in splice area.

Note:  
The 5'-0" steel pile splice sleeve shall be tightened enough to force out excess epoxy from around the circumference of the pile.

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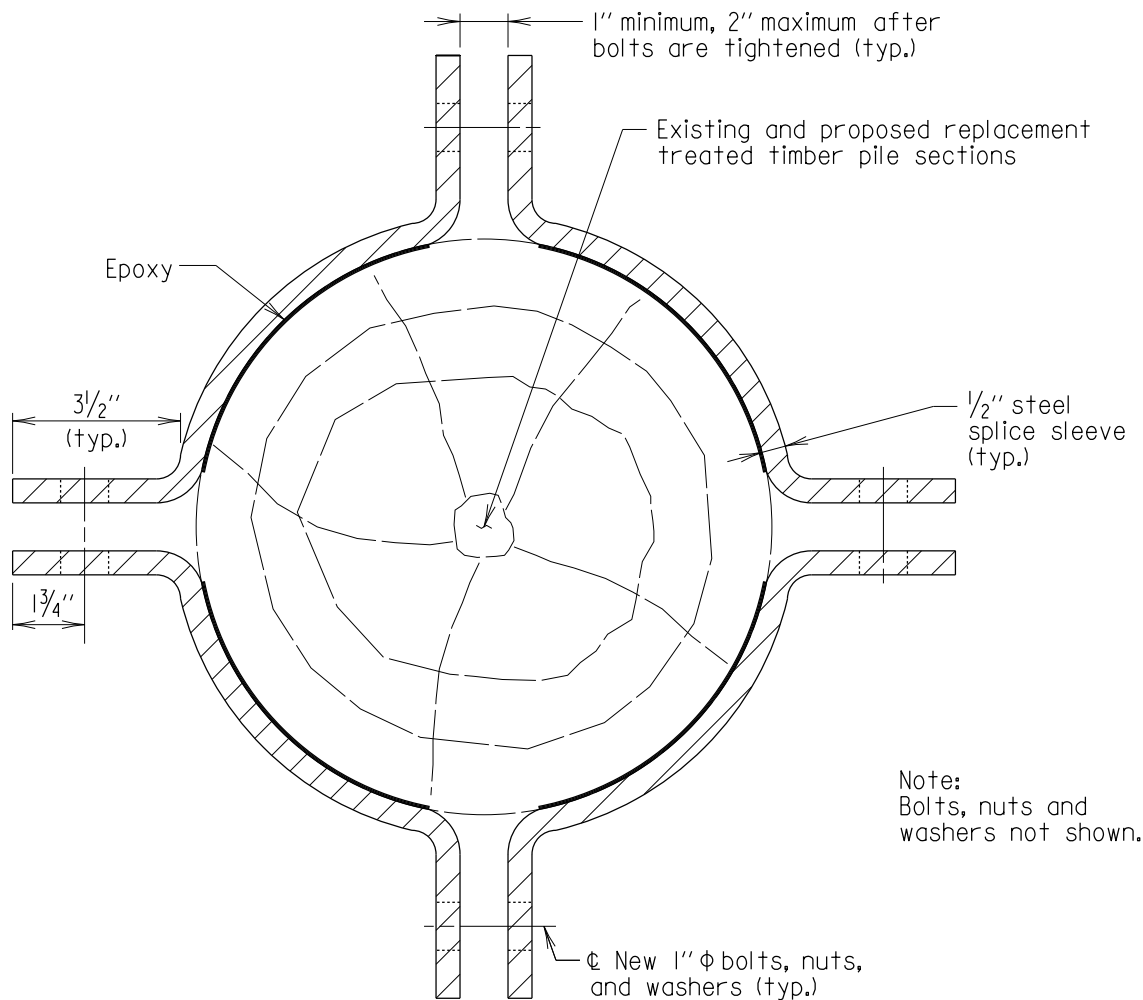
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SPLICE FOR CONNECTING EXISTING TIMBER  
PILING TO NEW TIMBER PILING

STANDARD NO. BR-SR(0.01)-95-304

SHEET 2 OF 8

STRUCTURAL REPAIRS



SECTION A-A (4 SECTION SPLICE ALTERNATE)

Scale: 3" = 1'-0"

Note:  
The four section splice can be used at any location.

Note:  
The 5'-0" steel pile splice sleeve shall be tightened enough to force out excess epoxy from around the circumference of the pile.

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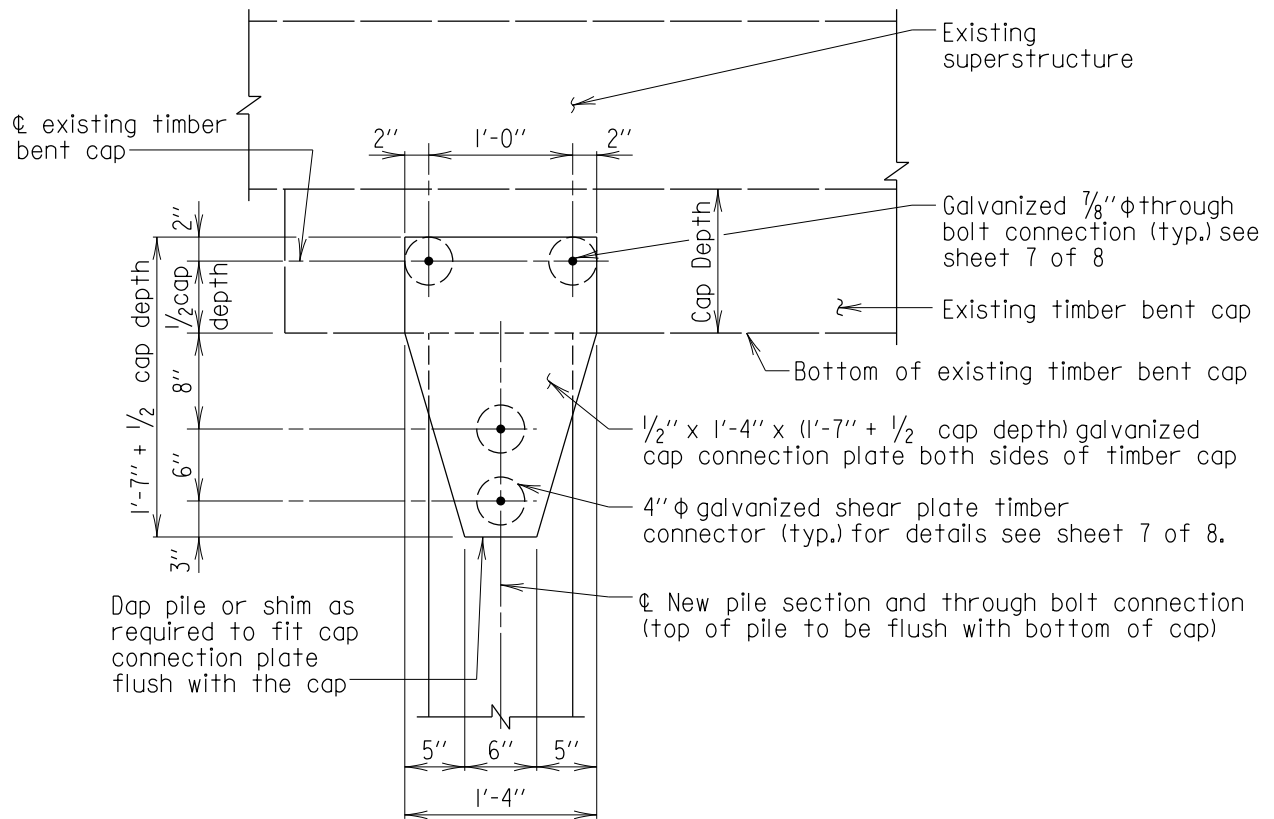
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SPLICE FOR CONNECTING EXISTING TIMBER  
PILING TO NEW TIMBER PILING

STANDARD NO. BR-SR(0.01)-95-304

SHEET 3 OF 8

STRUCTURAL REPAIRS



### PILE CONNECTION FOR NONSTRENGTHENED TIMBER CAPS

Scale:  $\frac{3}{4}" = 1'-0"$

#### Notes:

1. All field drilled holes in the piles shall have a compatible preservative treatment applied to them before bolting.
2. All steel plates, bolts, nuts, etc. shall be mechanically or hot dipped galvanized to conform with ASTM A 153.
3. Shims shall be galvanized ASTM A 709 Grade 36 steel.
4. All field drilled holes in the steel plates shall have a compatible galvanized touch up conforming to ASTM A 780 applied.

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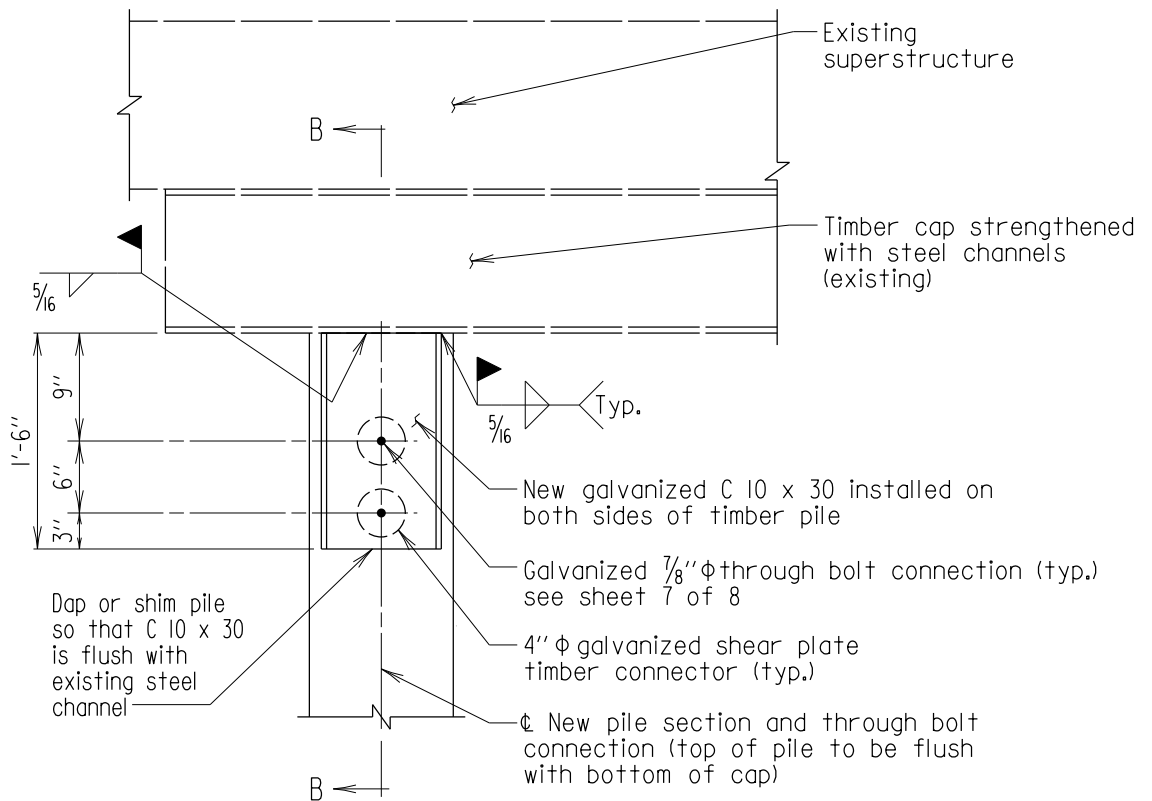
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### CONNECTION OF NEW TIMBER PILE SECTION TO EXISTING TIMBER CAP

STANDARD NO. BR-SR(0.01)-95-304

SHEET 4 OF 8

STRUCTURAL REPAIRS



### PILE CONNECTION FOR STEEL CHANNEL STRENGTHENED TIMBER CAPS

Scale:  $\frac{3}{4}" = 1'-0"$

#### Notes:

1. All steel plates, bolts, nuts, etc. shall be mechanically or hot dipped galvanized to conform to ASTM A 153.
2. Shims shall be galvanized ASTM A 709 Grade 36 steel.
3. This detail is not designed to transfer cap loads to the pile.
4. Areas of field welding and drilling shall be repaired with a galvanized touch up kit conforming to ASTM A 780.
5. All field drilled holes in the piles shall have a compatible preservative treatment applied to them before bolting.
6. For Section B-B see sheet 6 of 8.

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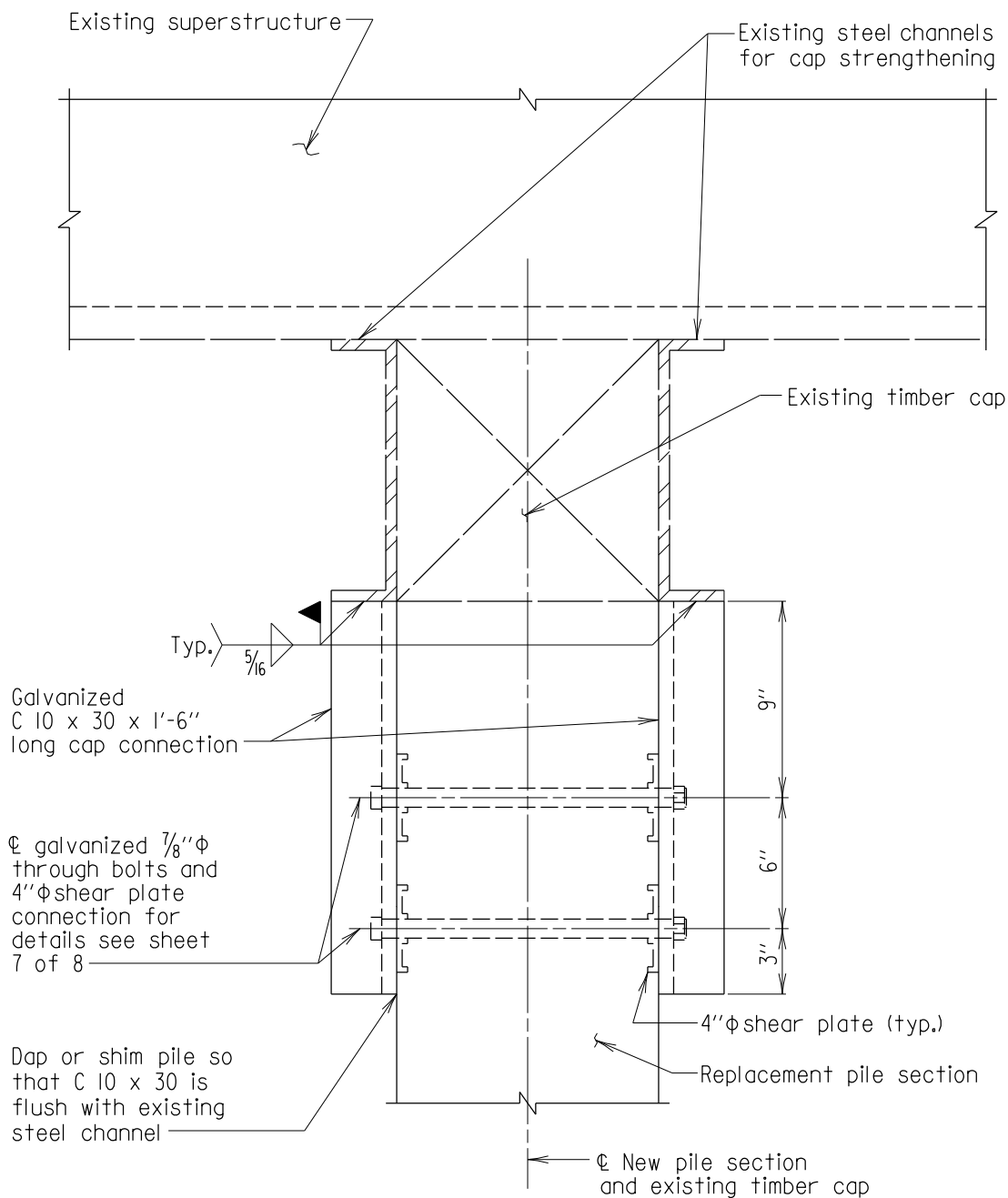
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### CONNECTION OF NEW TIMBER PILE SECTION TO EXISTING STEEL CHANNEL STRENGTHENED TIMBER CAP

STANDARD NO. BR-SR(0.01)-95-304

SHEET 5 OF 8

STRUCTURAL REPAIRS



### SECTION B-B

Scale: 1 1/2" = 1'-0"

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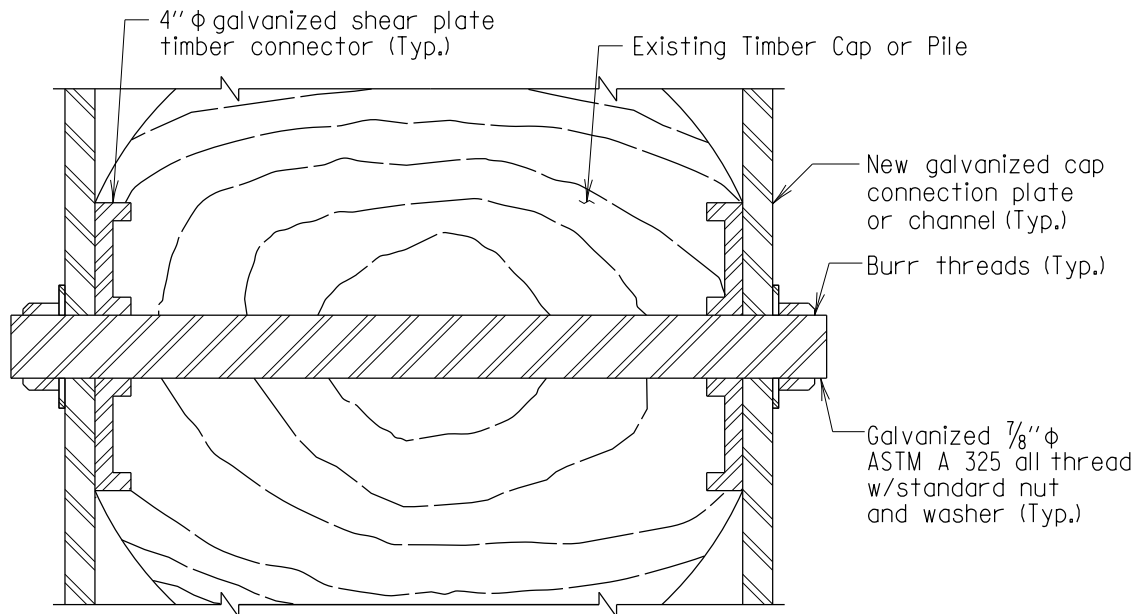
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CONNECTION OF NEW TIMBER PILE SECTION  
TO EXISTING STEEL CHANNEL STRENGTHENED  
TIMBER CAP

STANDARD NO. BR-SR(0.01)-95-304

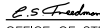
SHEET 6 OF 8

STRUCTURAL REPAIRS



TYPICAL THROUGH BOLT CONNECTION

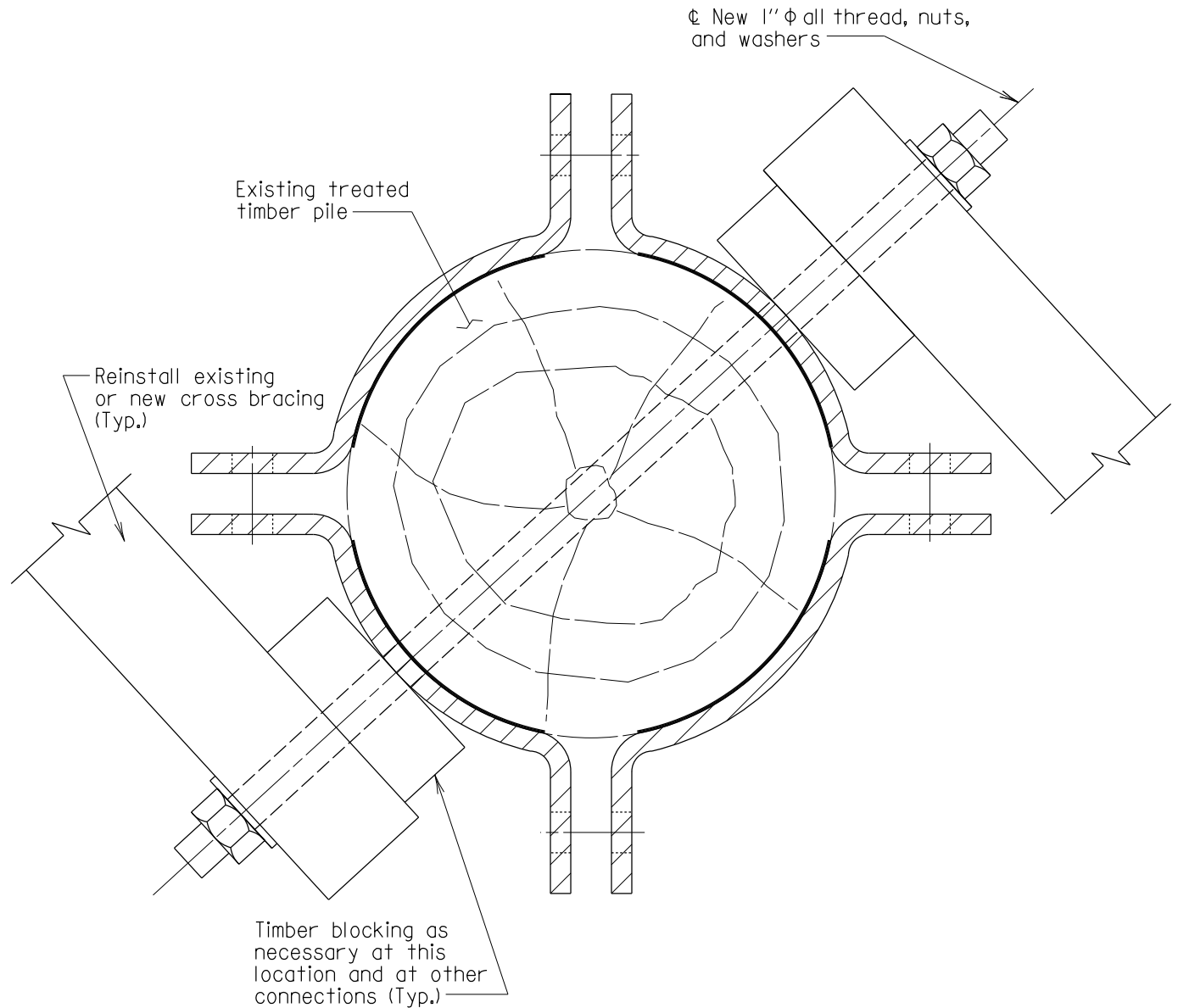
Scale:  $\frac{3}{8}" = 1"$

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CONNECTION OF NEW TIMBER PILE SECTION  
TO EXISTING STEEL CHANNEL STRENGTHENED  
TIMBER CAP

STANDARD NO. BR-SR(0.01)-95-304

SHEET 7 OF 8



#### 4 SECTION SPLICE CROSS BRACING DETAIL

Scale:  $1\frac{1}{2}'' = 1'-0''$

Note:  
Refer to the General Plan and  
Elevation to see whether new cross  
bracing is required and at which  
locations.

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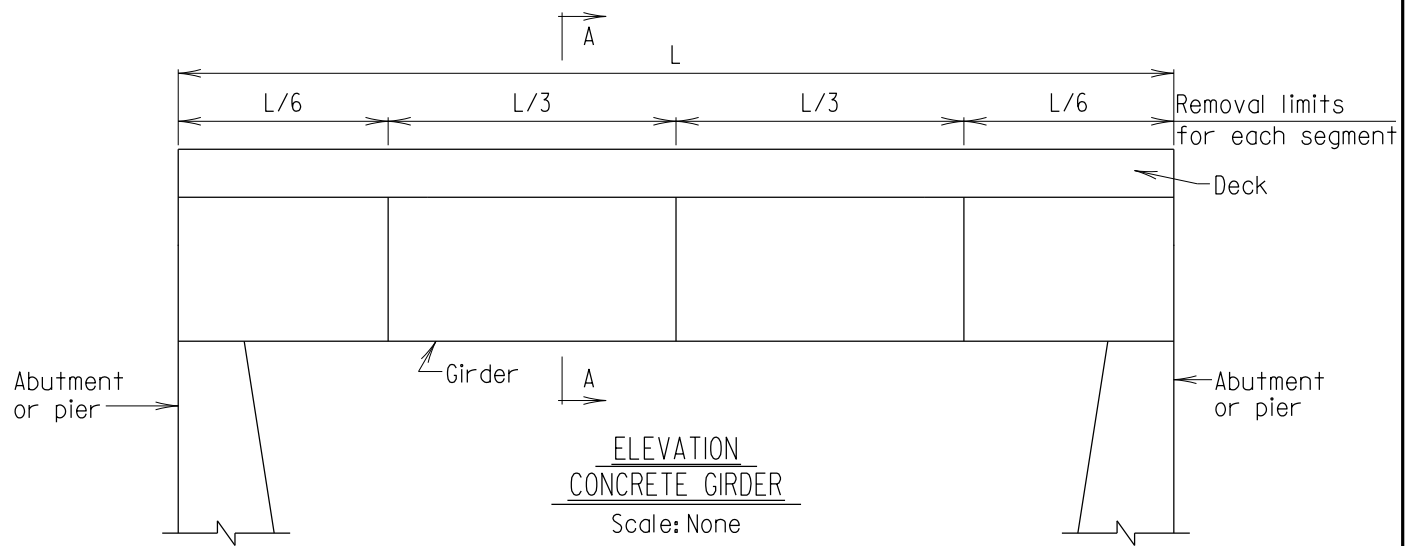
SPLICE FOR CONNECTING EXISTING TIMBER  
PILING TO NEW TIMBER PILING  
CROSS BRACING DETAILS

STANDARD NO. BR-SR(0.01)-95-304

SHEET 8 OF 8

STRUCTURAL REPAIRS





Note:  
E and F are original  
pier cap dimensions.

Remove to sound  
concrete to these  
lines, 6" maximum (typ.)

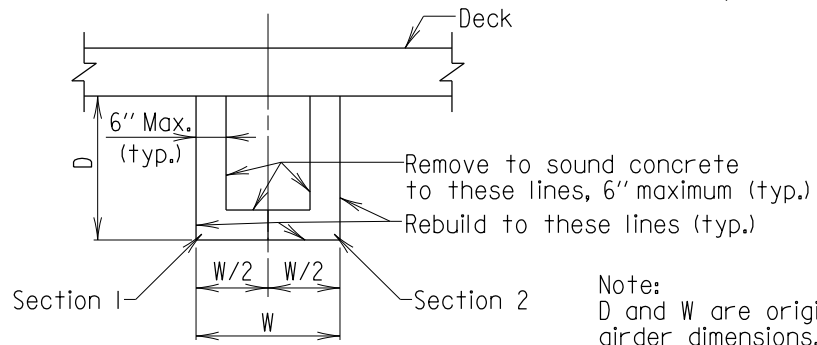
Rebuild to these  
lines (typ.)

6" Max. (typ.)

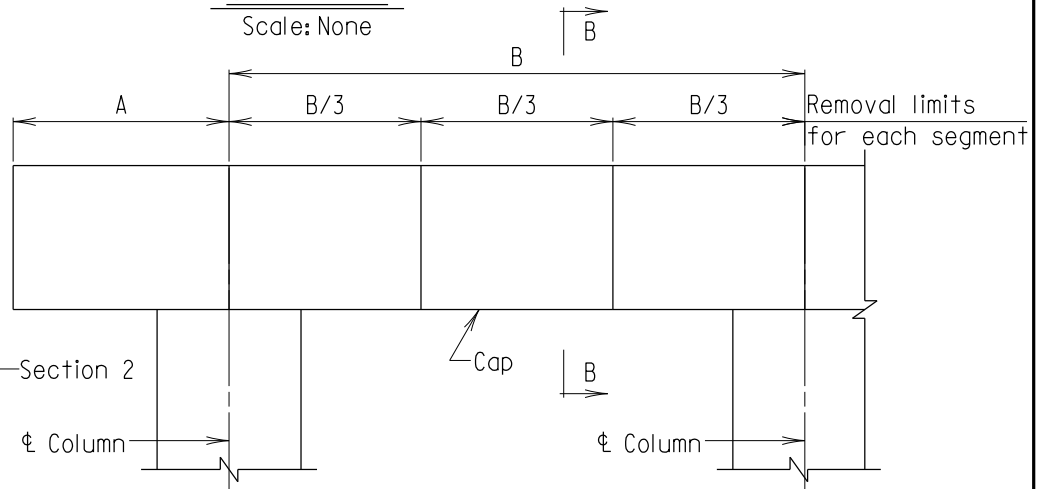
Section 1

SECTION B-B

Scale: None



Note:  
D and W are original  
girder dimensions.



Notes:

- Concrete elements are divided into segments. Work to be performed on sections as provided below.
- Refer to Section 420 and 421.
- Contractor shall stage the work so that the worse sections are repaired first.
- Contractor shall not work on more than two nonadjacent sections on each pier cap, column, or girder at one time.
- Contractor shall wait 72 hours after completing repairs to a section before chipping adjacent sections, however he may perform work on other bridge elements.
- Contractor shall stop removing deteriorated concrete when a maximum depth of 6 in. is reached. The Engineer shall immediately notify the Office of Structures if more removal seems necessary.
- Existing reinforcing steel not shown.
- For column repair details, see sheet 2 of 2.

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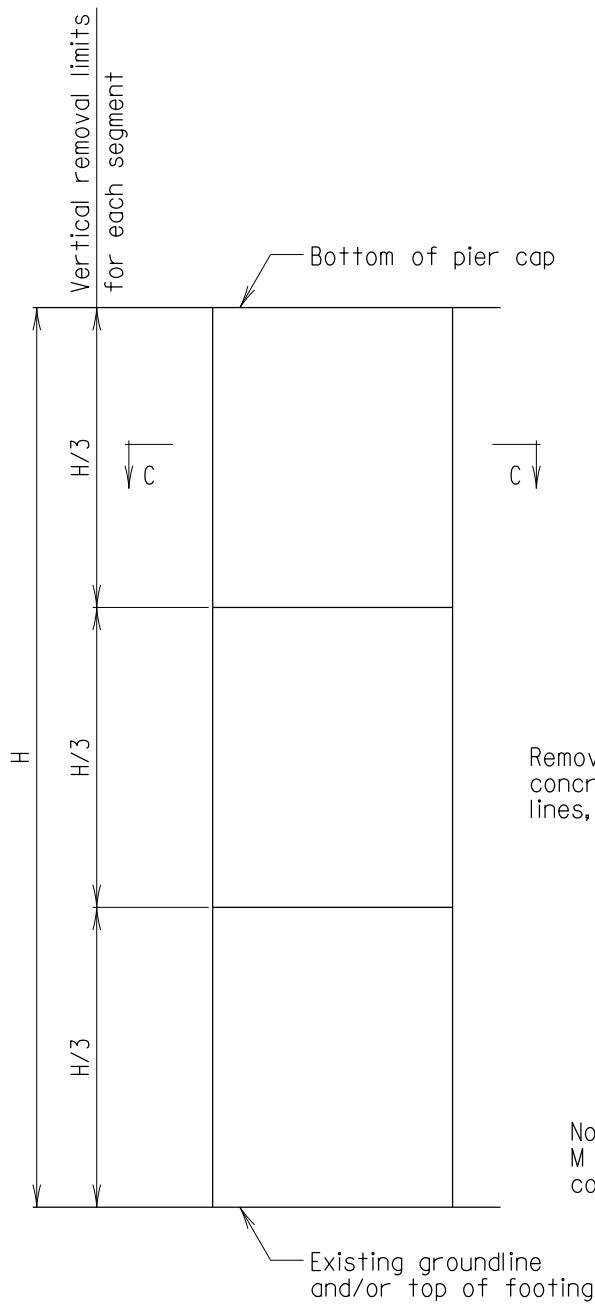
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CAST-IN-PLACE CONCRETE  
SEQUENCE AND REPAIR DETAILS

STANDARD NO. BR-SR(0.02)-95-305

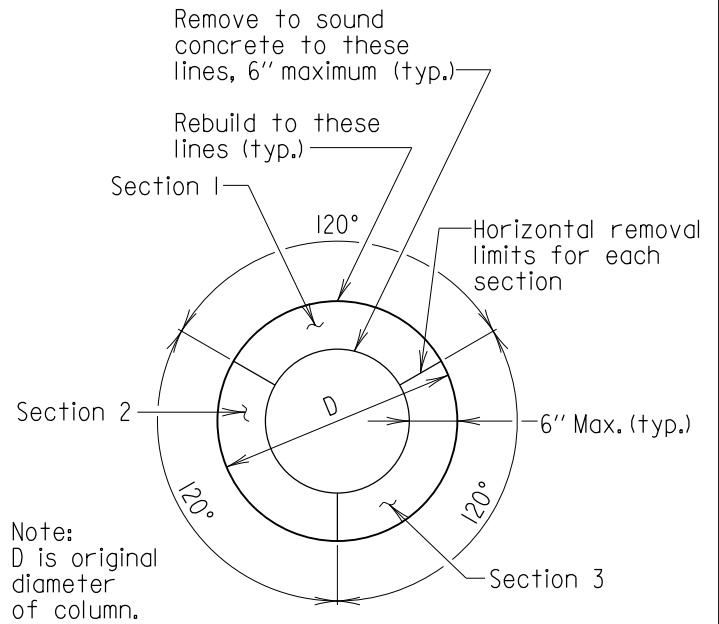
SHEET 1 OF 2

STRUCTURAL REPAIRS

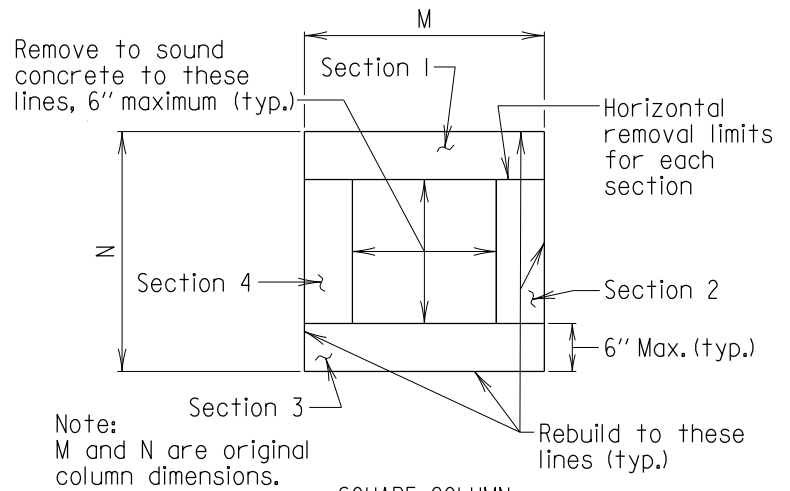


ELEVATION  
PIER COLUMN

Scale: None



CIRCULAR COLUMN



SQUARE COLUMN

SECTION C-C

Scale: None

Notes:

1. When height of columns is more than 18 ft., the Contractor will be restricted to 6 ft. segments.
2. Refer to Section 420 and 421.
3. For notes, see sheet 1 of 2.

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CAST-IN-PLACE CONCRETE  
SEQUENCE AND REPAIR DETAILS

STANDARD NO. BR-SR(0.02)-95-305

SHEET 2 OF 2

## DESIGN GUIDELINES FOR JACKING BEAMS

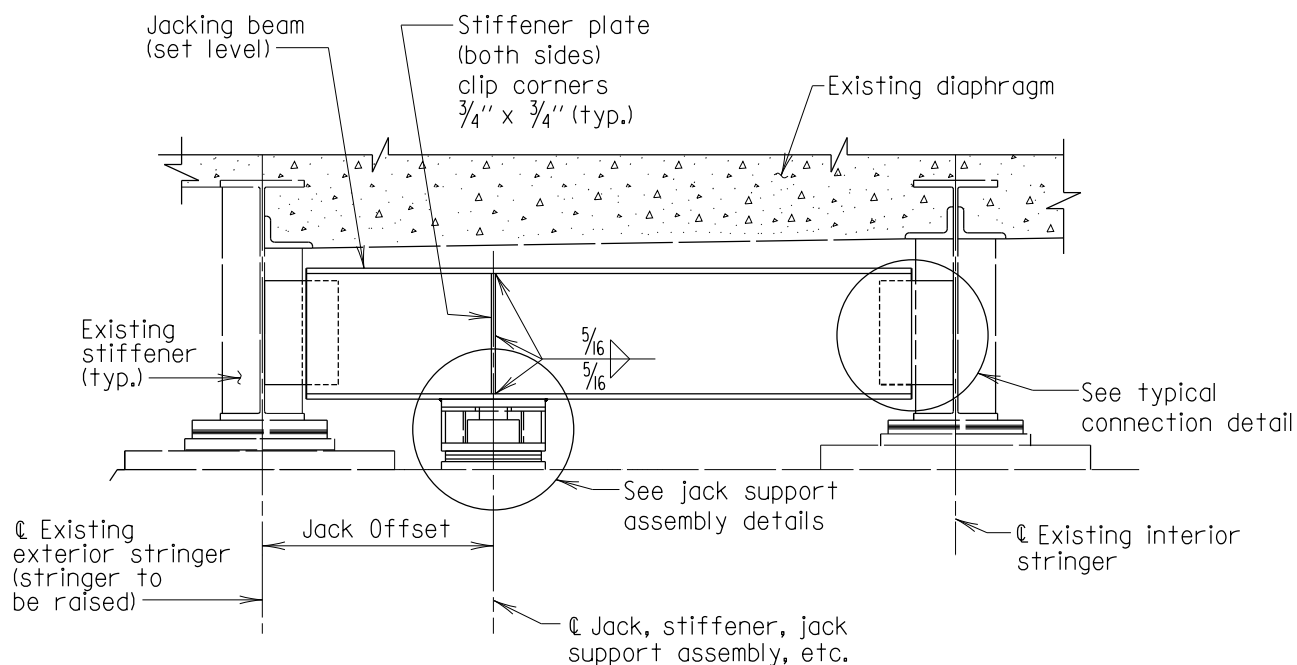
- 1) The temporary jacking system is to be designed at operating stress levels.
- 2) A five percent increase to the dead load beam reaction is required for deck stiffness.
- 3) Bolts shall be ASTM A 325 with the threads included in the shear plane if possible. The connection shall be designed in bearing with the reduced root area. ASTM A 490 bolts are acceptable.
- 4) Minimum stiffener and connection plate thickness shall be  $\frac{1}{2}$ ".
- 5) Designers should attempt to minimize the number of different jacking systems for the bridge by designing a system that will work in multiple locations.
- 6) Minimum fillet weld size shall be  $\frac{5}{16}$ ".
- 7) Avoid bent connection plates where possible. If the skew angle does not allow placing straight connection plates from the existing stiffener to the web, attach the connection plate full height to the existing web and design it as a stiffener. Place it far enough from the existing stiffener to allow welding the connection plate to the web and still have full bearing under the jacking system.
- 8) The jack stand can only accomodate a jack with a diameter of 6" or less. Most jacks greater than 75 tons will require a different stand.
- 9) The possibility of shifting traffic off of the stringer to be jacked should be discussed with the ADE-Traffic. This would allow designing for only dead load.
- 10) When designing a jacking beam the designer may want to start with the following trial sections:

* LOAD (X)	BOLTS	BEAM	CONNECTION PLATE
$X \leq 35K$	3 - $\frac{7}{8}$ " $\phi$ A 325	W 12 x 26	$\frac{1}{2}$ " x 9"
$35K < X \leq 45K$	3 - 1" $\phi$ A 325	W 14 x 26	$\frac{1}{2}$ " x 11"
$45K < X \leq 60K$	4 - 1" $\phi$ A 325	W 18 x 35	$\frac{1}{2}$ " x 14 $\frac{1}{2}$ "
$60K < X \leq 80K$	4 - 1" $\phi$ A 490	W 18 x 35	$\frac{1}{2}$ " x 18"

\* Load (X) is dead load and live load plus impact at the bolts

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### EXTERIOR - JACKING BEAM

Scale : None

JACKING BEAM TABLE		
	Size:	Location:
Jacking Beam		
Steel Grade		
Stiffener Plate Size		
Jack Offset		
Maximum Jack Force		
Minimum Section Modulus		
Minimum Cross Sectional Web Area		
Minimum Web Thickness		

#### Notes:

1. Jacking beams do not have to be new, but shall be in good condition.
2. The jack shall not be used to support load during bearing repairs.
3. Jacking beams shall be placed level unless otherwise noted.
4. The Contractor has the option of submitting another method of jacking to the Engineer for approval. The design shall be done by a PE registered in Md.
5. Jacking beams shall be kept low to minimize height of stacked plates or the HP column jack support.
6. Anchor bolt nuts may need to be loosened at the exterior and adjacent interior stringers to allow the stringer to rise.
7. Stringers shall not be raised more than  $\frac{1}{8}$ " above its existing elevation.

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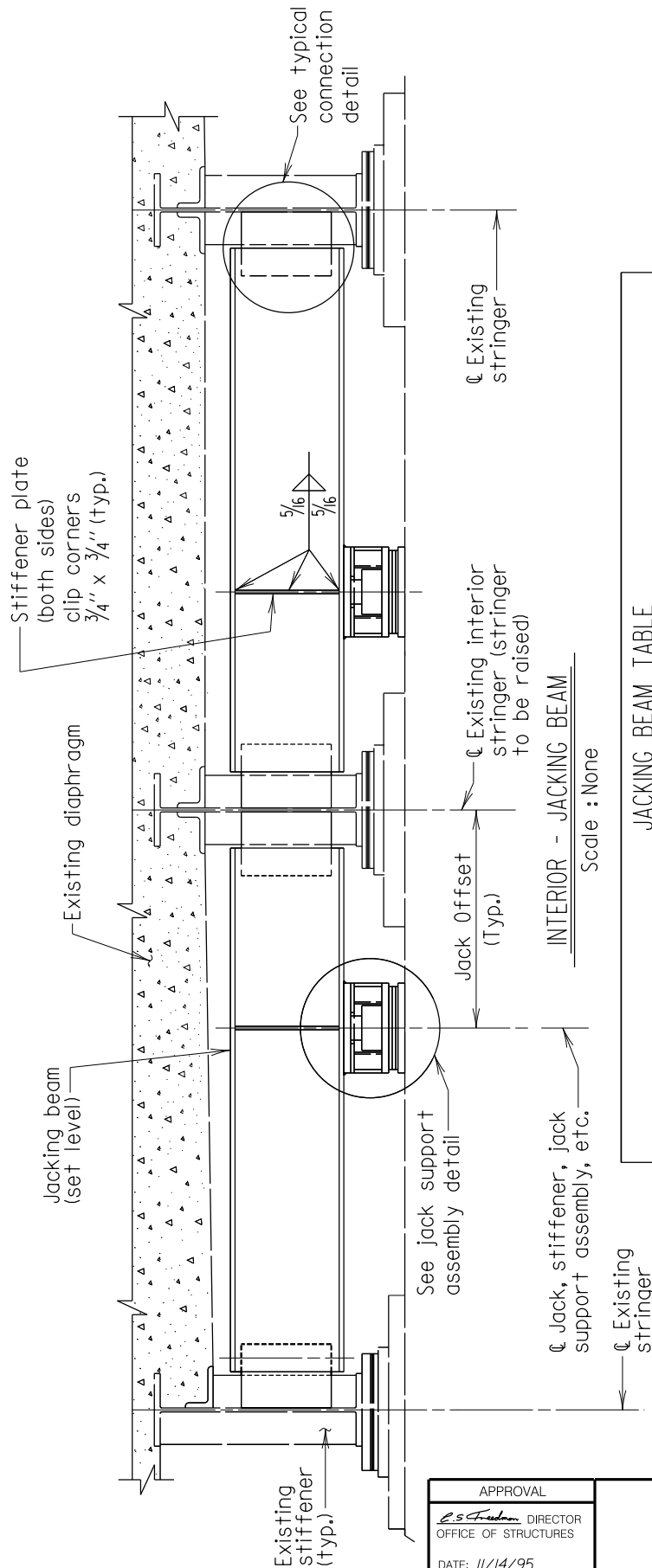
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### EXTERIOR BEAM JACKING DETAILS

STANDARD NO. BR-SR(0.04)-95-307

SHEET 1 OF 6



JACKING BEAM TABLE	
Size:	Location:
Jacking Beam	
Steel Grade	
Stiffener Plate Size	
Jack Offset	
Maximum Jack Force	
Minimum Section Modulus	
Minimum Cross Sectional Web Area	
Minimum Web Thickness	

**Notes:**

1. Jacking beams do not have to be new, but shall be in good condition.
2. The jack shall not be used to support load during bearing repairs.
3. Jacking beams shall be placed level unless otherwise noted.
4. The Contractor has the option of submitting another method of jacking to the Engineer for approval. The design shall be done by a PE registered in Md.
5. Jacking beams shall be kept low to minimize height of stacked plates or the HP column jack support.
6. Anchor bolt nuts may need to be loosened at the exterior and adjacent interior stringers to allow the stringer to rise.
7. Stringers shall not be raised more than 1/8" above its existing elevation.

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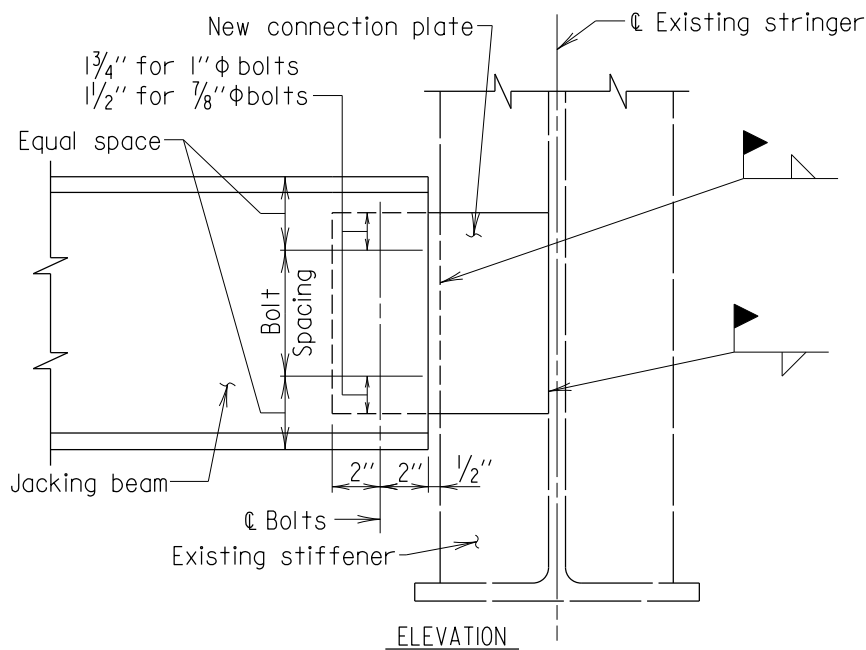
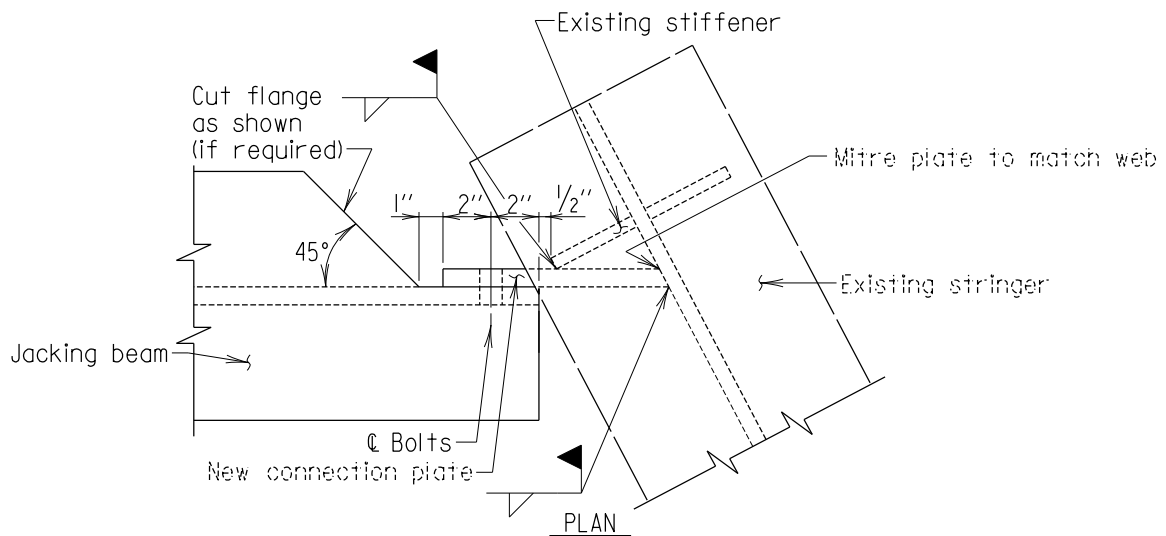
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INTERIOR BEAM JACKING DETAILS

STANDARD NO. BR-SR(0.04)-95-307

SHEET 2 OF 6



### TYPICAL SKEWED CONNECTION DETAIL

Scale : None

CONNECTION DETAILS		
	Materials:	Location:
Connection Plate Size		
Connection Plate Weld		
Number of Bolts		
Bolt (Type and Size)		
Bolt Spacing c/c		
Existing Stiffener Plate Size		
Steel Grade		

#### Notes:

Any steel that has been welded to the existing bridge shall remain in place. The repaired area and any other areas damaged shall be repaired in conformance with 436.

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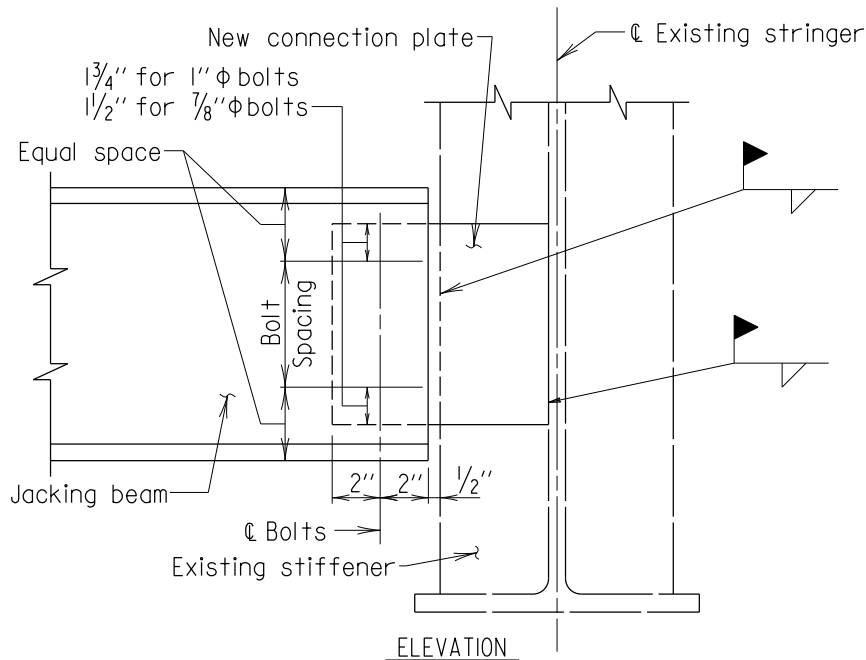
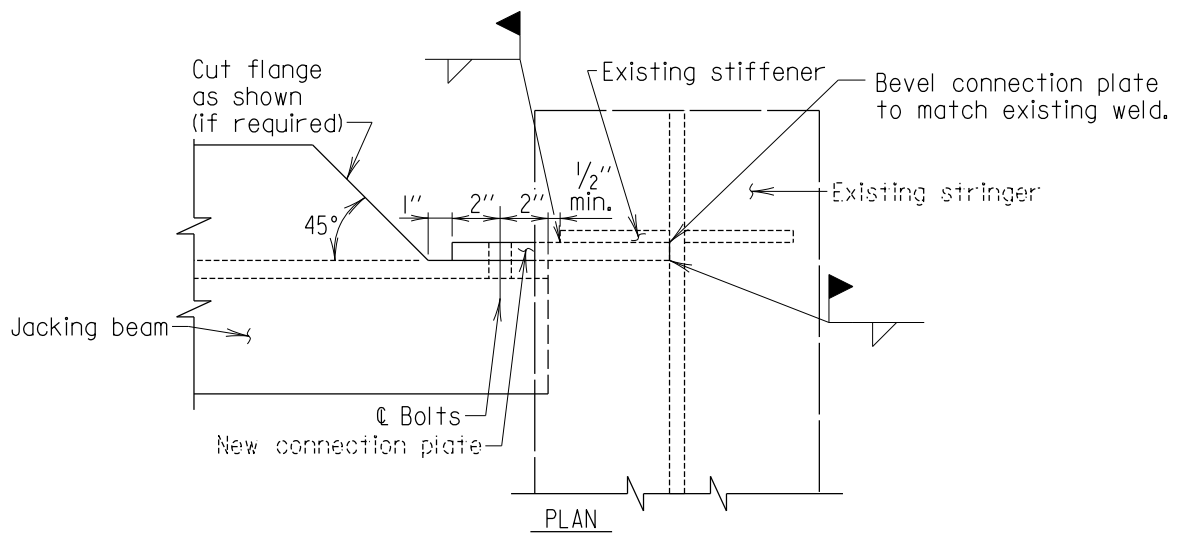
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### SKEWED CONNECTION DETAILS

STANDARD NO. BR-SR(0.04)-95-307

SHEET 3 OF 6



TYPICAL 90° CONNECTION DETAIL

Scale : None

CONNECTION DETAILS		
	Materials:	Location:
Connection Plate Size		
Connection Plate Weld		
Number of Bolts		
Bolt (Type and Size)		
Bolt Spacing c/c		
Existing Stiffener Plate Size		
Steel Grade		

**Notes:**

Any steel that has been welded to the existing bridge shall remain in place. The repaired area and any other areas damaged shall be repaired in conformance with 413.03.15.

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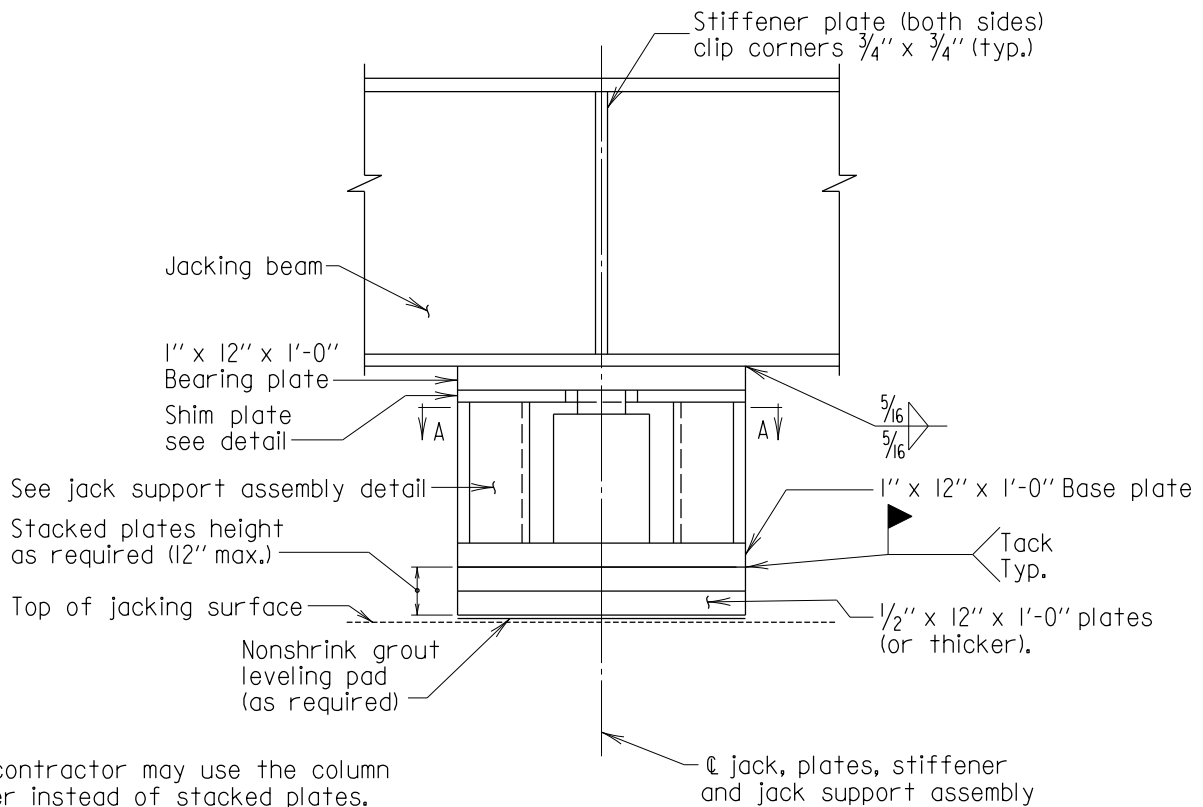
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90° CONNECTION DETAILS

STANDARD NO. BR-SR(0.04)-95-307

SHEET 4 OF 6



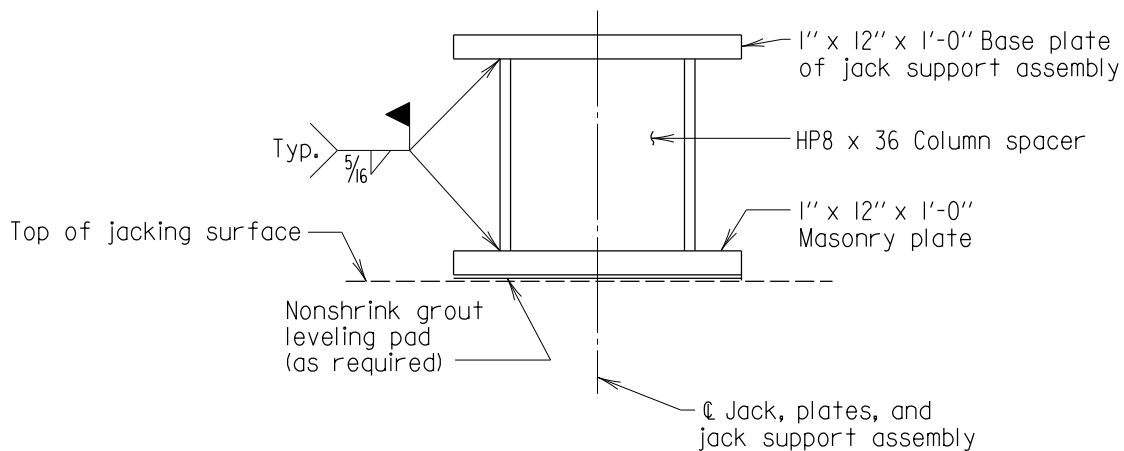
**Note:**

The contractor may use the column spacer instead of stacked plates. The column spacer shall be used for heights greater than 12" to a maximum of 5'-0" high.

ELEVATION

JACK SUPPORT USING STACKED PLATES

Scale : None



ELEVATION

ALTERNATE COLUMN SPACER DETAIL

Scale : None

**Notes:**

1. Minimum thickness of the grout leveling pad shall be as recommended by manufacturer.
2. Jack shall be centered under jacking beam web and stiffeners.
3. Stacked plates shall not exceed 12" high.
4. HP8 x 36 column spacer shall not exceed 5'-0" high.
5. All material to be ASTM A 709 Grade 36.

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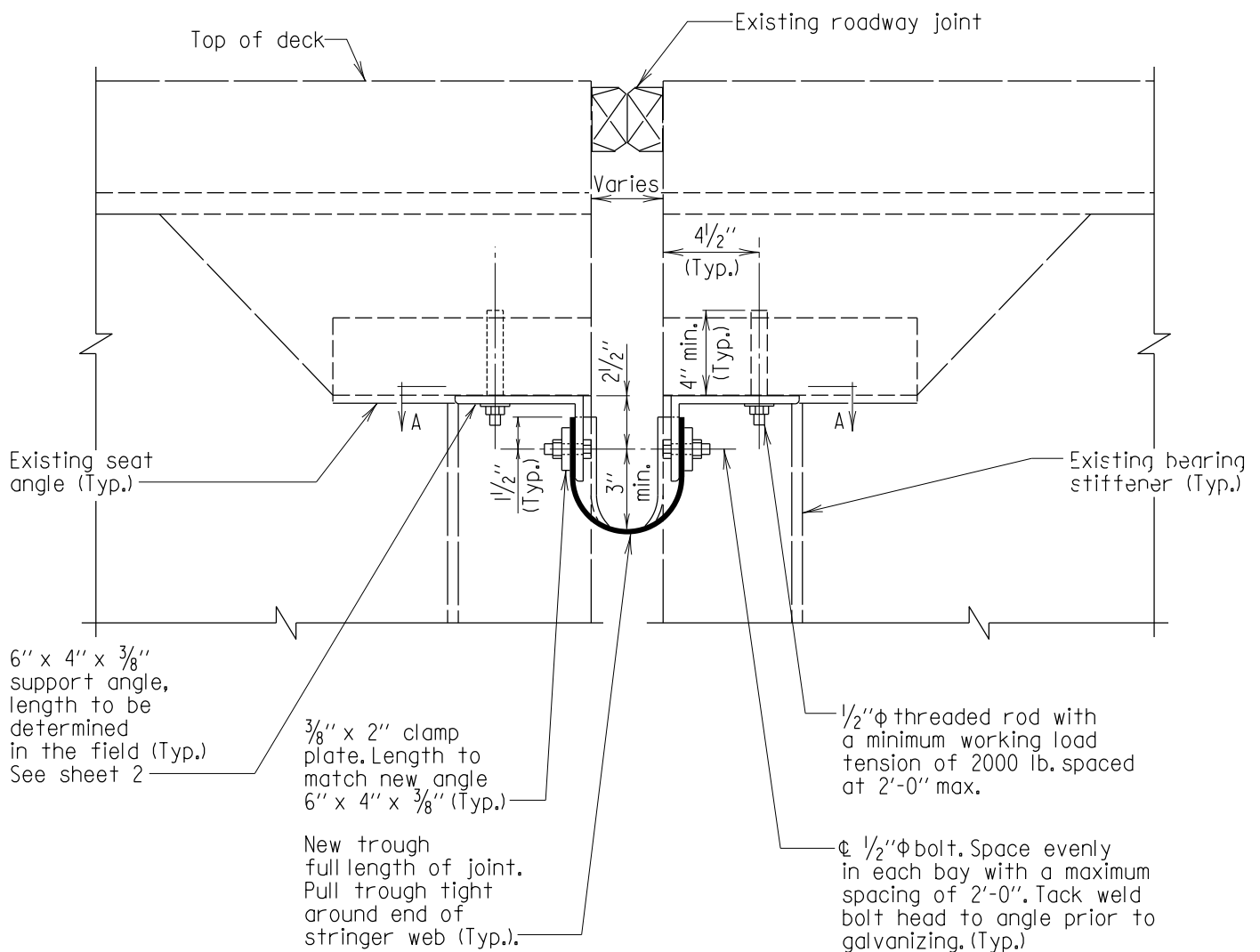
JACK SUPPORT ASSEMBLY

STANDARD NO. BR-SR(0.04)-95-307

SHEET 5 OF 6







TROUGH DETAIL BETWEEN BEAMS AT PIER

Scale: 1 1/2" = 1'-0"

**Notes:**

1. All steel shall be galvanized ASTM A 709 Grade 36.
2. Trough shall conform to 911.11.
3. Trough cross slope shall be a minimum of 1" per foot for finger joints. All other joints shall follow the grade of the end diaphragms or 1/4" per foot slope whichever is greater.
4. All hardware shall be stainless steel Type 304.
5. Drilled holes for threaded rods shall be 1/2" larger.
6. Grout shall conform to 902.11(c).
7. Fiberglass shall conform to 921.11.

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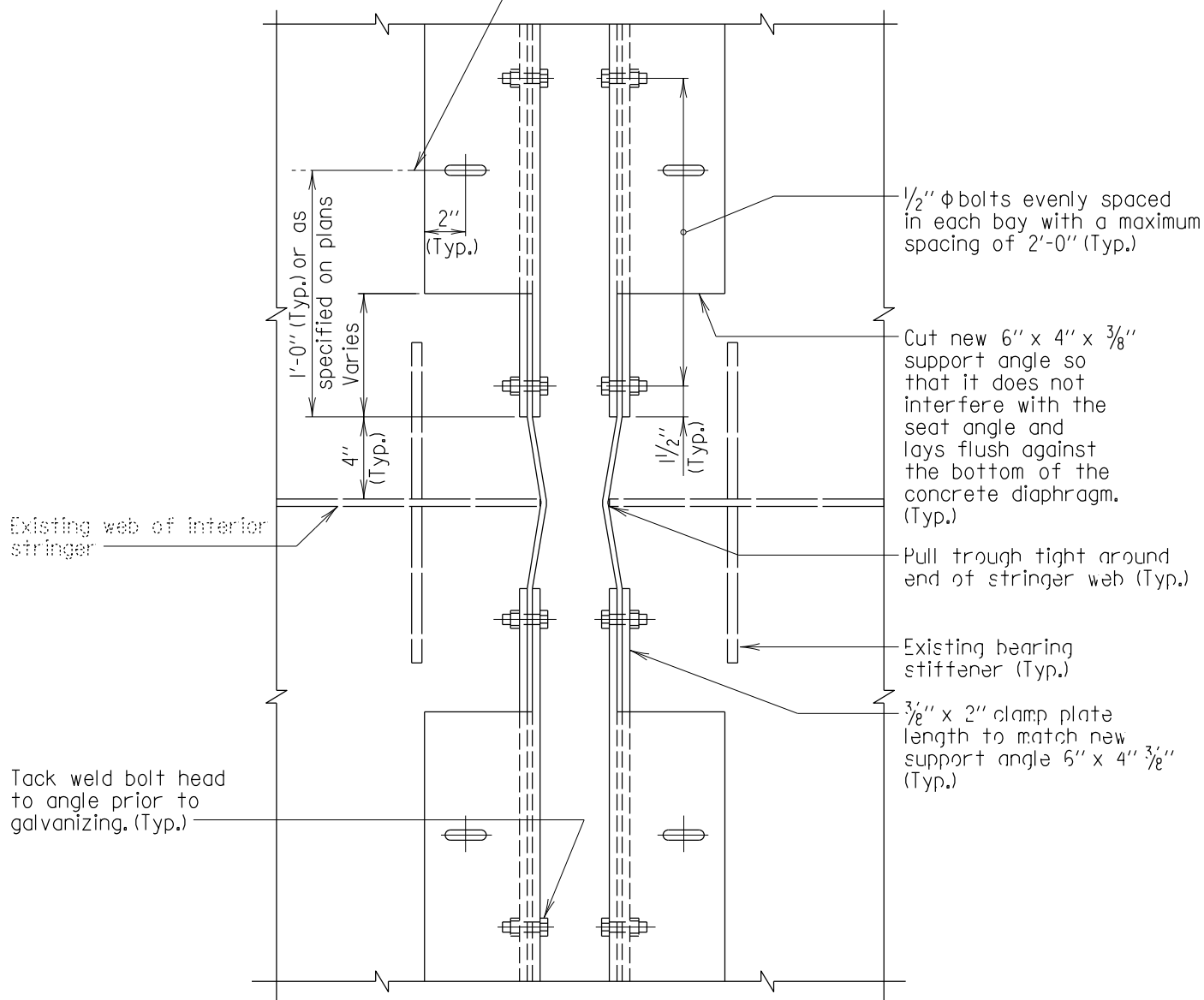
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DRAINAGE TROUGH DETAIL AT PIER  
FOR EXISTING STRUCTURE

STANDARD NO. BR-SR(0.05)-95-308

SHEET 1 OF 8

③  $\frac{1}{16}$ " x 2" long slotted hole located at a maximum spacing of 2'-0" and 1'-0" from each end of the support angle (Typ.).



### SECTION A-A

Scale:  $1\frac{1}{2}$ " = 1'-0"

Note:  
Existing seat angle not shown for clarity.

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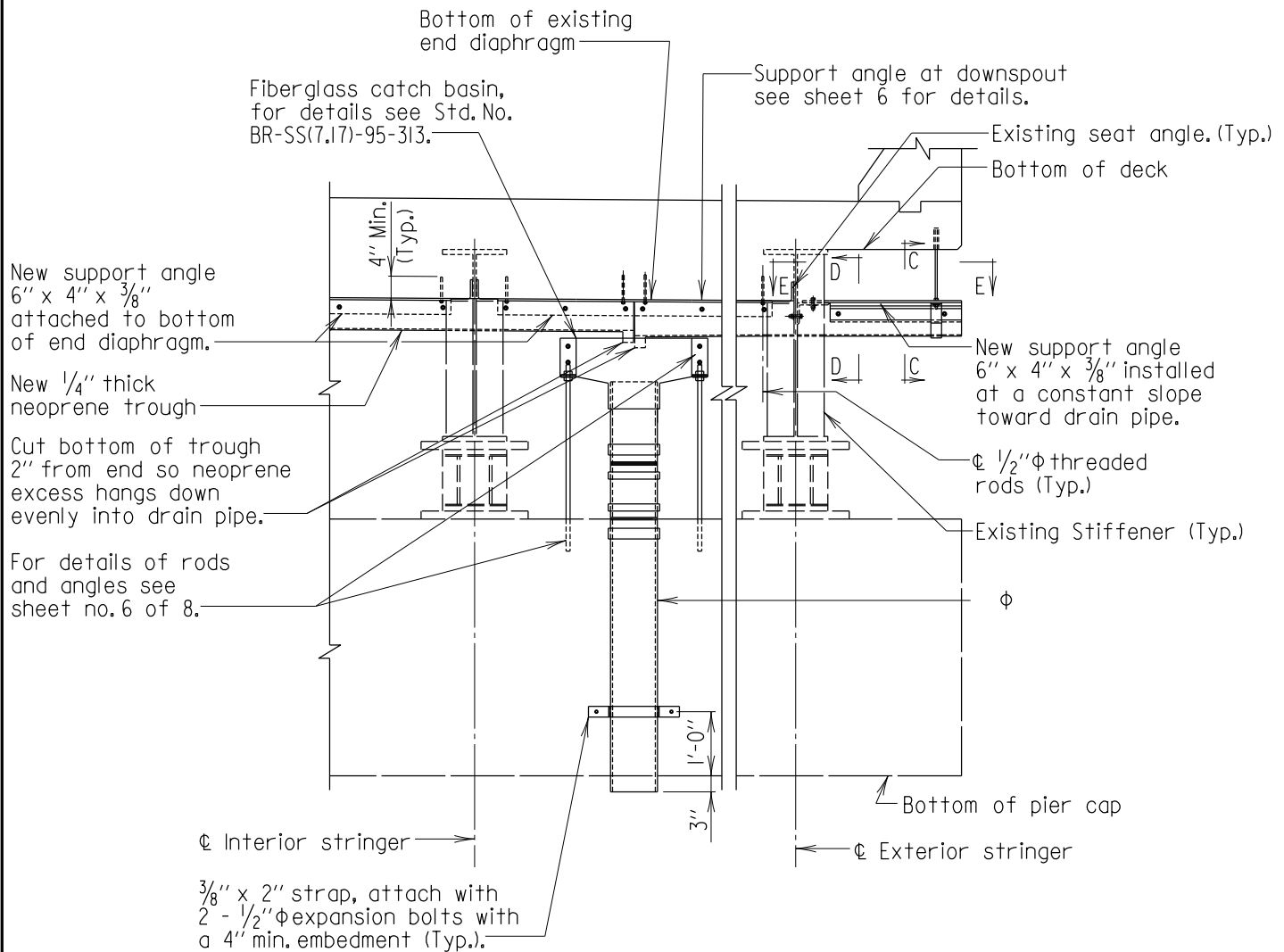
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DRAINAGE TROUGH DETAIL AT PIER  
FOR EXISTING STRUCTURE

STANDARD NO. BR-SR(0.05)-95-308

SHEET 2 OF 8

STRUCTURAL REPAIRS



### DOWNSPOUT DETAIL BETWEEN BEAMS AT PIER

Scale: 3/8" = 1'-0"

#### Note:

1. For location of downspout refer to the General Plan and Elevation.
2. Refer to M(6.04)-80-119 for splash block requirements.

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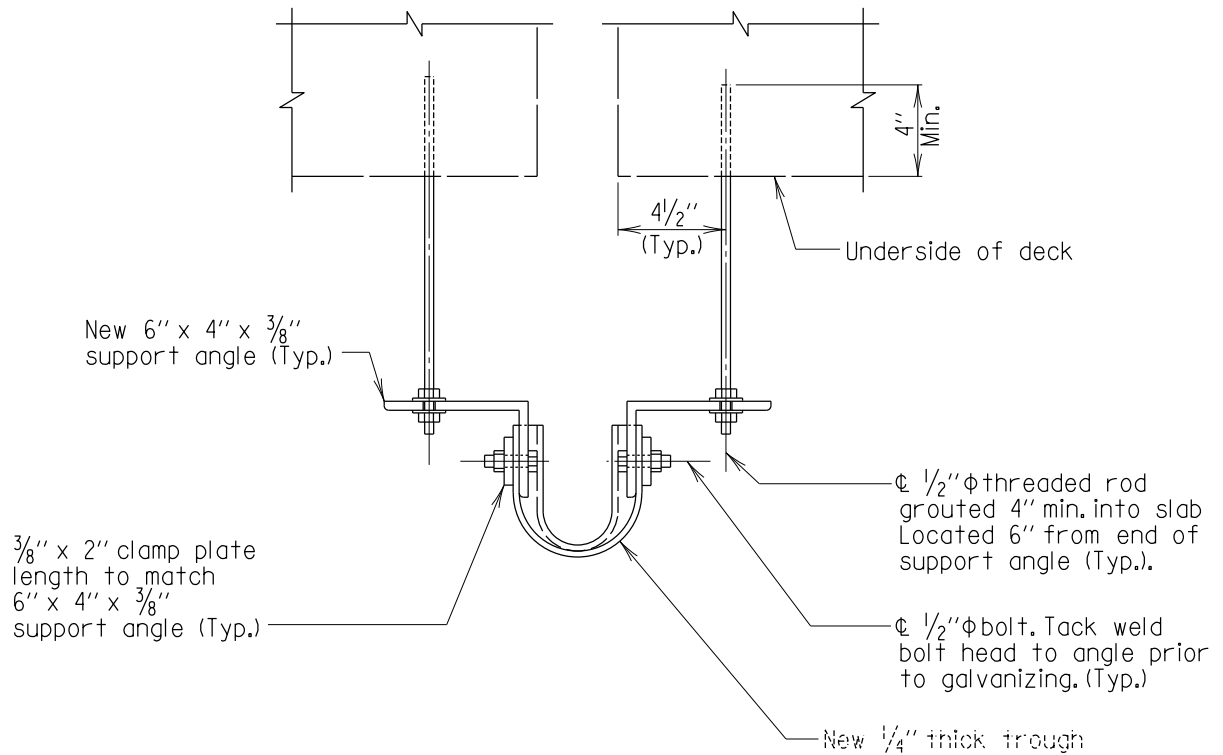
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### DRAINAGE TROUGH DETAIL AT PIER FOR EXISTING STRUCTURE

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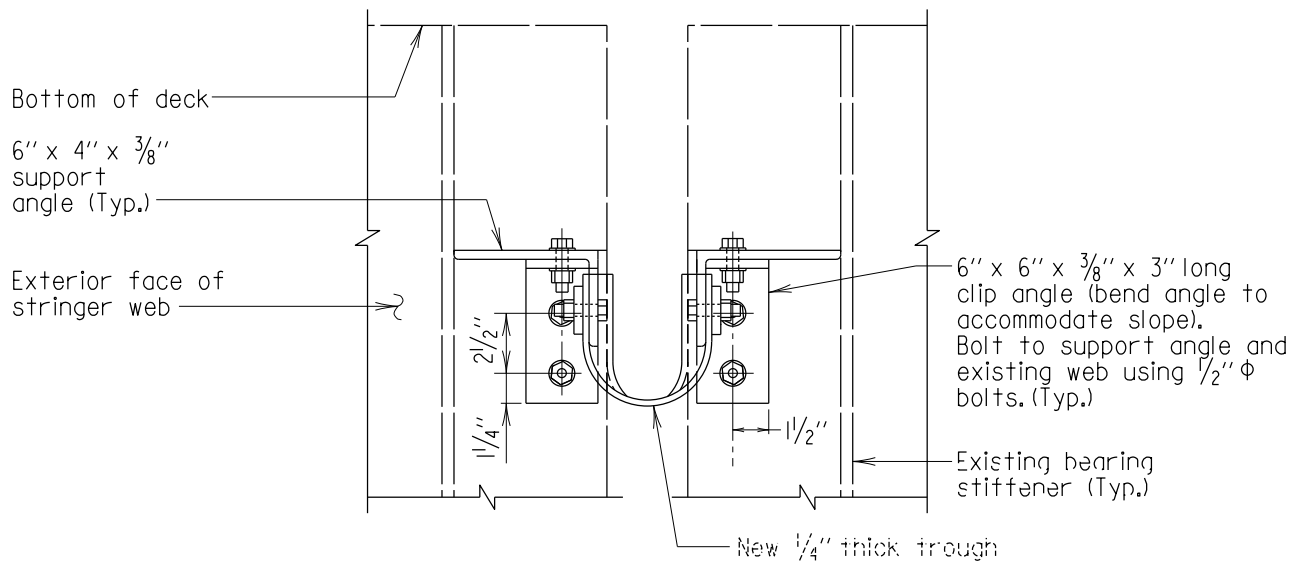
SHEET 3 OF 8

STRUCTURAL REPAIRS



### SECTION C-C

Scale: 1  $\frac{1}{2}$ " = 1'-0"



### SECTION D-D

Scale: 1  $\frac{1}{2}$ " = 1'-0"

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DRAINAGE TROUGH DETAIL AT PIER  
FOR EXISTING STRUCTURE

STANDARD NO. BR-SR(0.05)-95-308

SHEET 4 OF 8

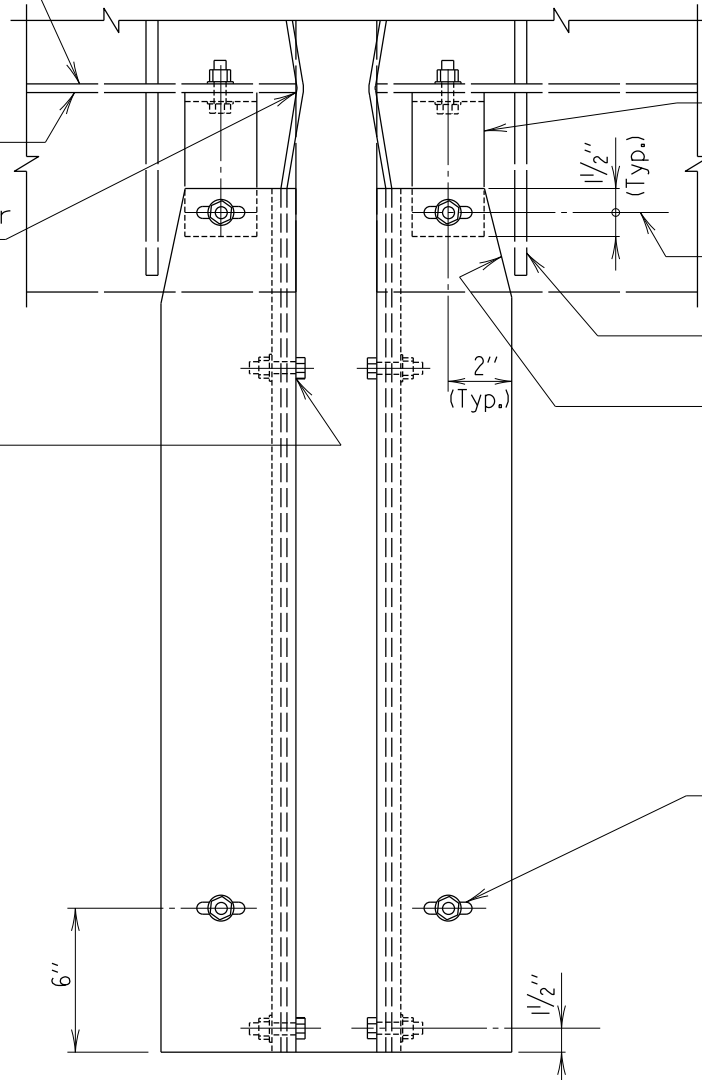
STRUCTURAL REPAIRS

Existing web  
of exterior  
stringer

Existing exterior  
face of stringer web

Pull trough tight  
around end of stringer  
web. (Typ.)

Tack weld bolt head  
to angle prior to  
galvanizing. (Typ.)



6" x 6" x  $\frac{3}{8}$ " x 3" long galvanized  
clip angle (bend to accommodate  
slope). Bolt to support angle  
and existing web using  
 $\frac{1}{2}$ "  $\phi$  bolts.

$\phi$   $\frac{11}{16}$ " x 2" long slotted  
hole (typ.)

Existing bearing  
stiffener (Typ.)

Cope new 6" x 4" x  $\frac{3}{8}$ "  
support angle to  
avoid existing bearing  
stiffener where  
necessary.

$\frac{1}{2}$ "  $\phi$  threaded rod  
grouted 4" min. into  
slab

SECTION E-E  
Scale:  $1\frac{1}{2}$ " = 1'-0"

Note:  
Existing seat angle not shown for  
clarity.

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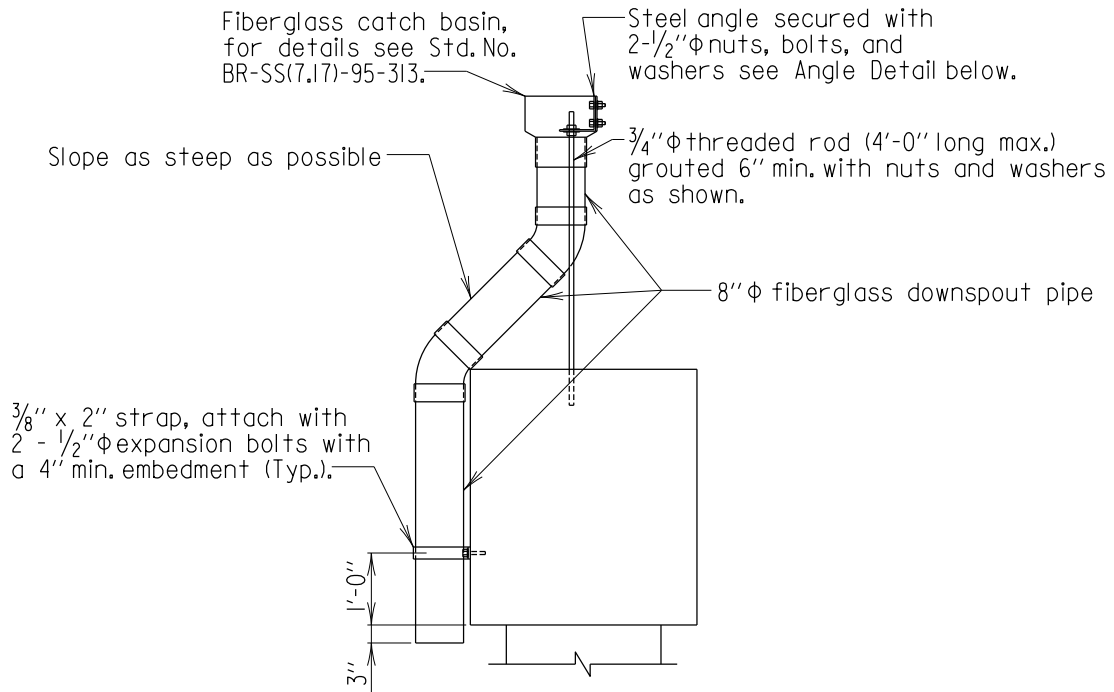
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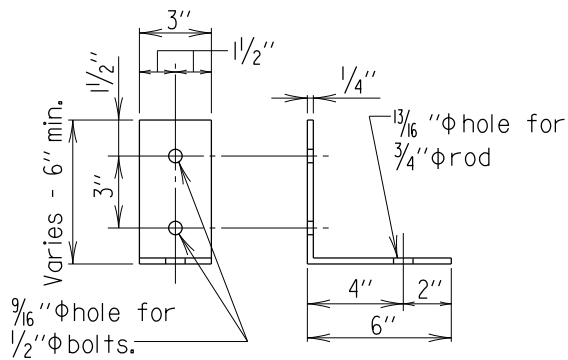
SHEET 5 OF 8

STRUCTURAL REPAIRS



### DOWNSPOUT DETAIL FOR PIER CAPS

Scale: 3/8" = 1'-0"



### ANGLE DETAIL

Scale: 1 1/2" = 1'-0"

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DRAINAGE TROUGH DETAIL AT PIER  
FOR EXISTING STRUCTURE

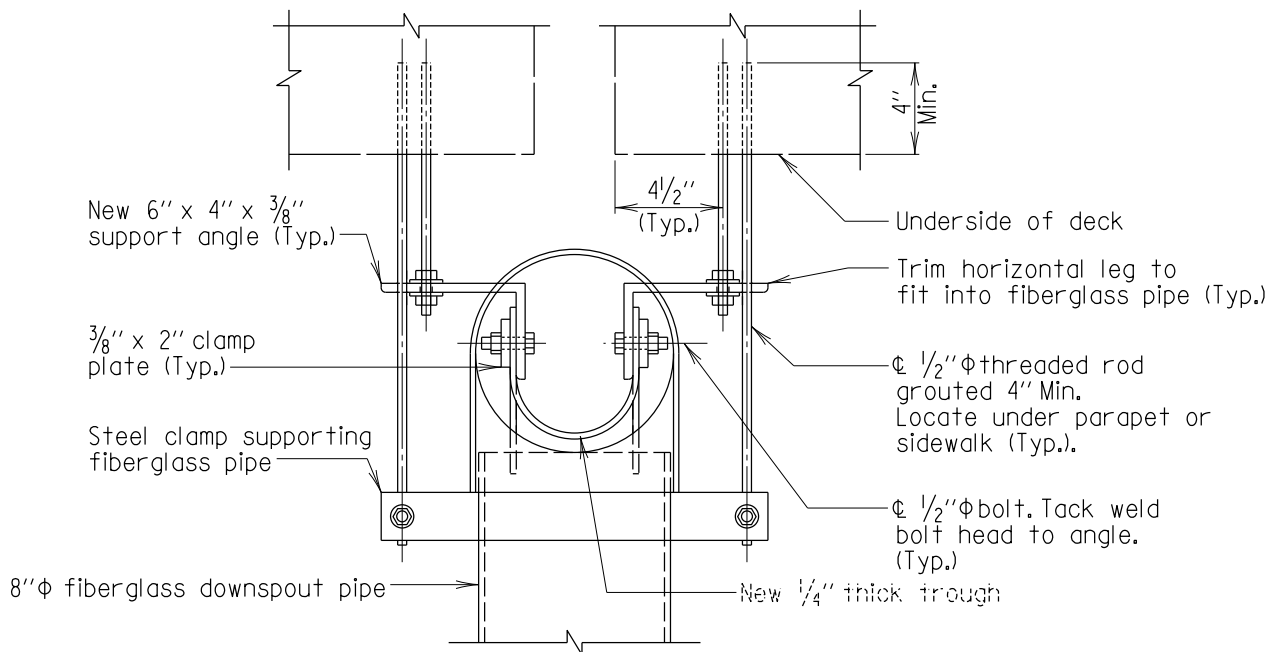
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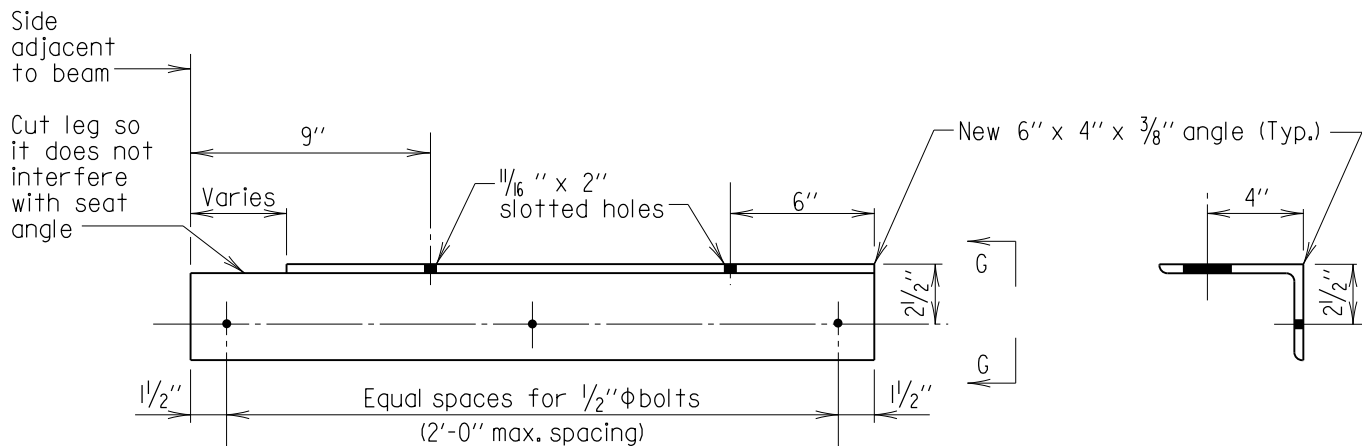
STRUCTURAL REPAIRS







SECTION H-H  
Scale: 1 1/2" = 1'-0"



SUPPORT ANGLE AT DOWNSPOUT  
Scale: 1 1/2" = 1'-0"

VIEW G-G  
Scale: 1 1/2" = 1'-0"

Note:  
Length of support angle to be determined in the field.

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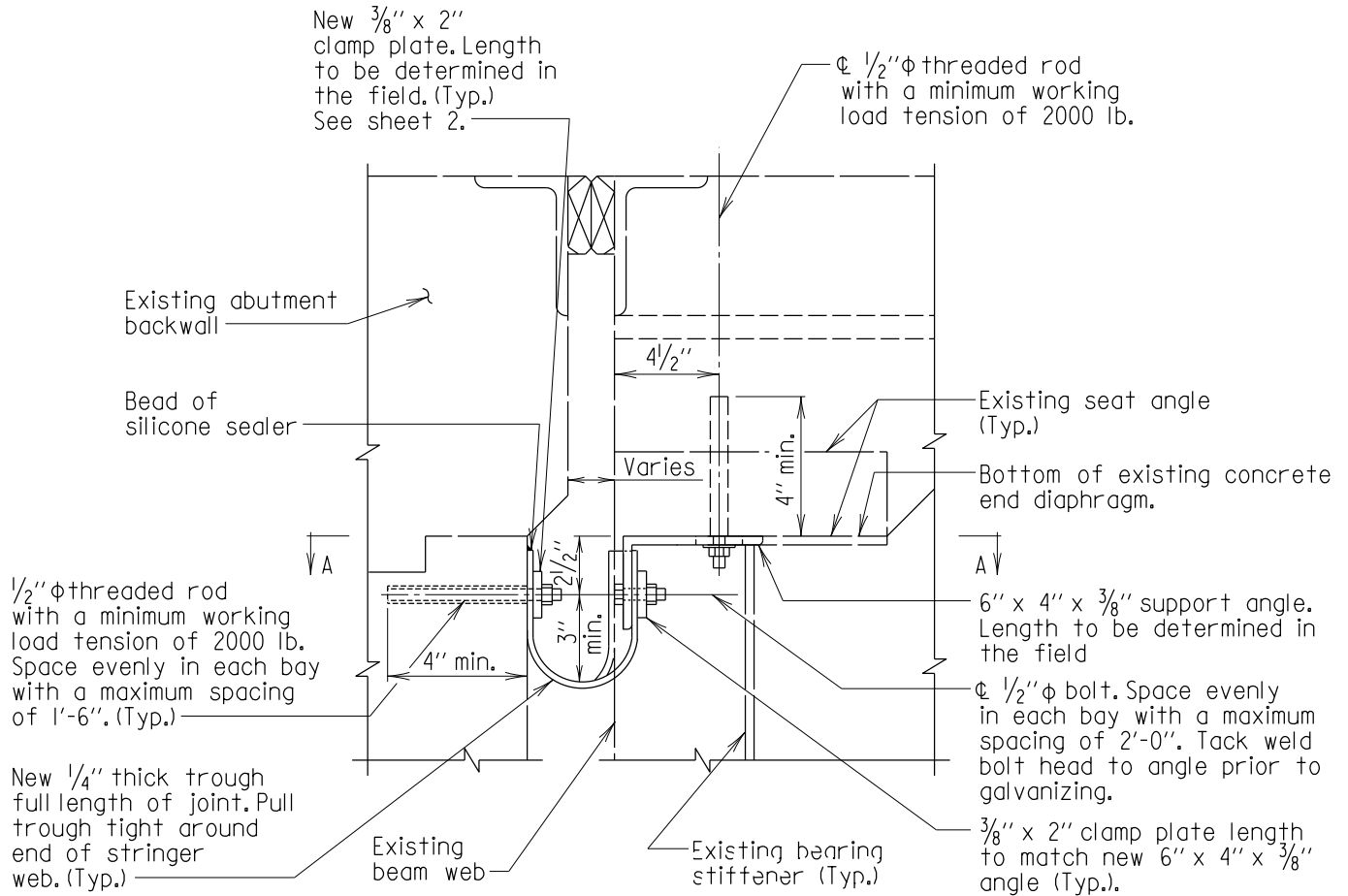
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DRAINAGE TROUGH DETAIL AT PIER  
FOR EXISTING STRUCTURE

STANDARD NO. BR-SR(0.05)-95-308

SHEET 8 OF 8

STRUCTURAL REPAIRS



### TROUGH DETAIL BETWEEN BEAMS AT ABUTMENT

Scale:  $1\frac{1}{2}'' = 1'-0''$

#### Notes:

1. All steel shall be galvanized ASTM A 709 Grade 36.
2. Trough shall conform to 911.11 or 911.12.
3. Trough cross slope shall be a minimum of 1" per foot for finger joints. All other joints shall follow the grade of the end diaphragms or  $\frac{1}{4}''$  per foot slope whichever is greater.
4. All hardware shall be stainless steel Type 304.
5. Drilled holes for threaded rods shall be  $\frac{1}{2}''$  larger.
6. Grout shall conform to 902.11(c).
7. Fiberglass shall conform to 921.11.
8. Silicone sealer shall conform to 911.01.01.

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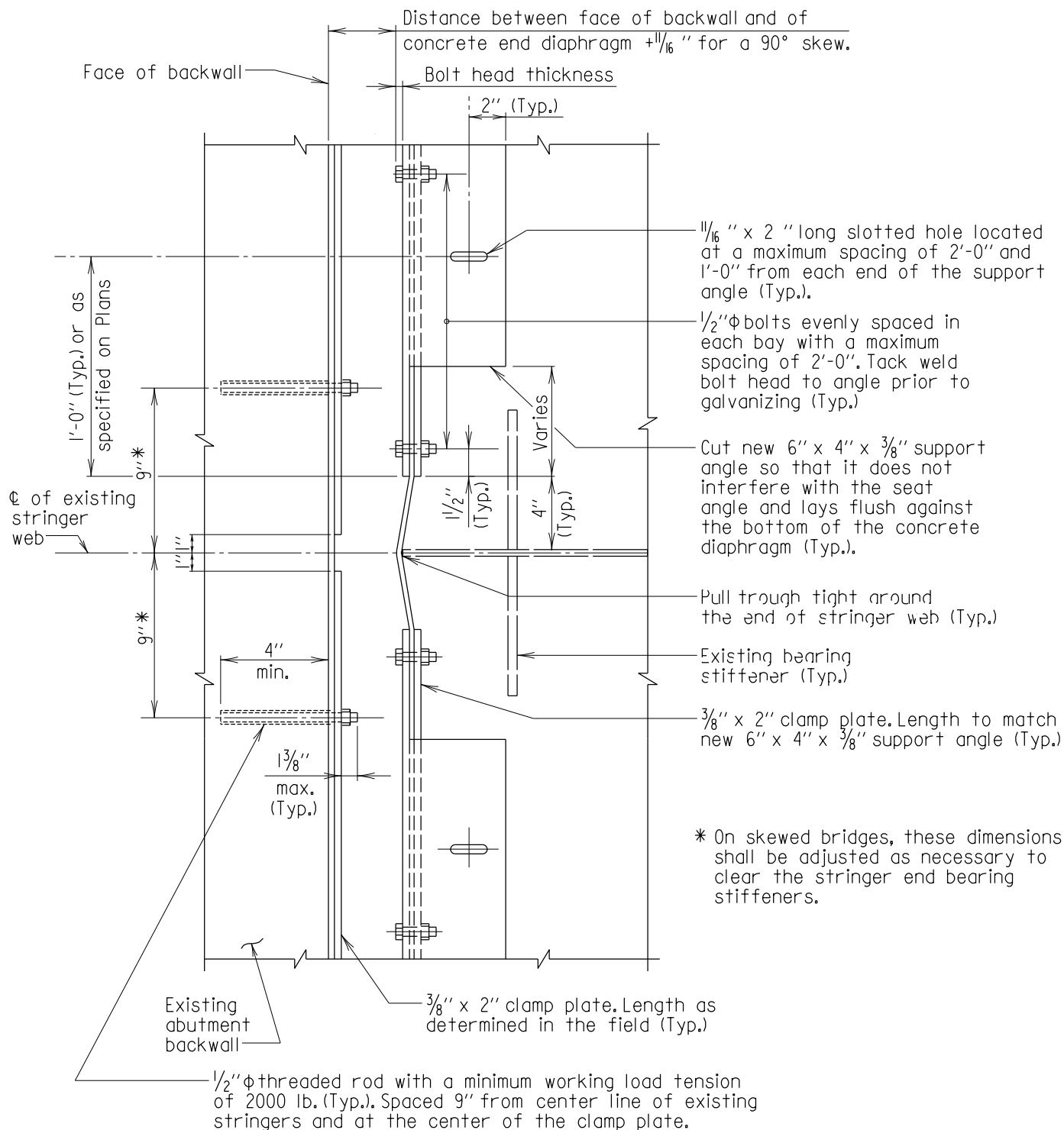
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### DRAINAGE TROUGH DETAIL AT ABUTMENT FOR EXISTING STRUCTURE

STANDARD NO. BR-SR(0.06)-95-309

SHEET 1 OF 7



### SECTION A-A

Scale: 1 1/2" = 1'-0"

Note:  
Existing seat angle not shown for clarity.

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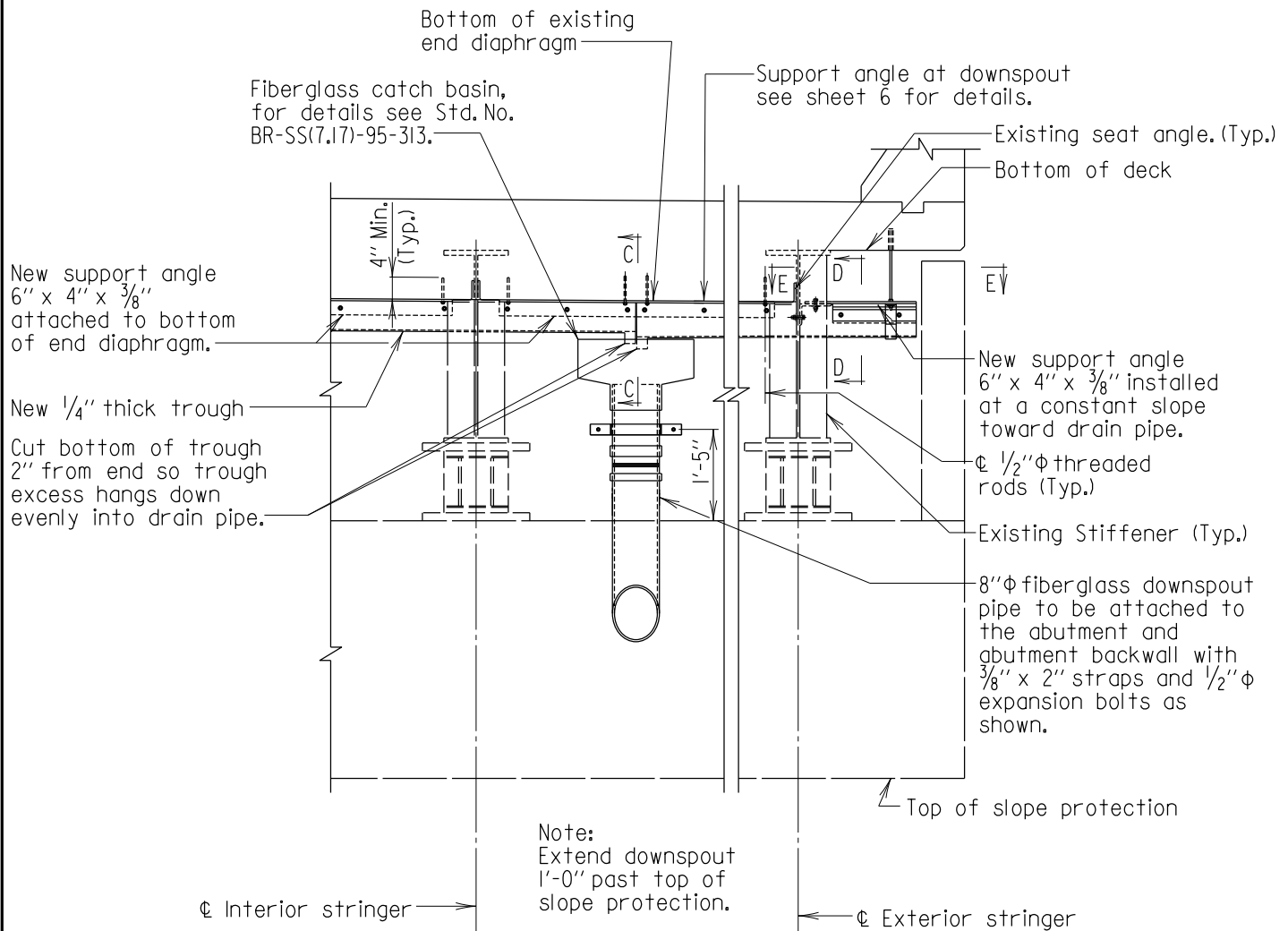
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DRAINAGE TROUGH DETAIL AT ABUTMENT  
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STANDARD NO. BR-SR(0.06)-95-309

SHEET 2 OF 7

STRUCTURAL REPAIRS



### DOWNSPOUT DETAIL BETWEEN BEAMS AT ABUTMENT

Scale:  $\frac{3}{8}" = 1'-0"$

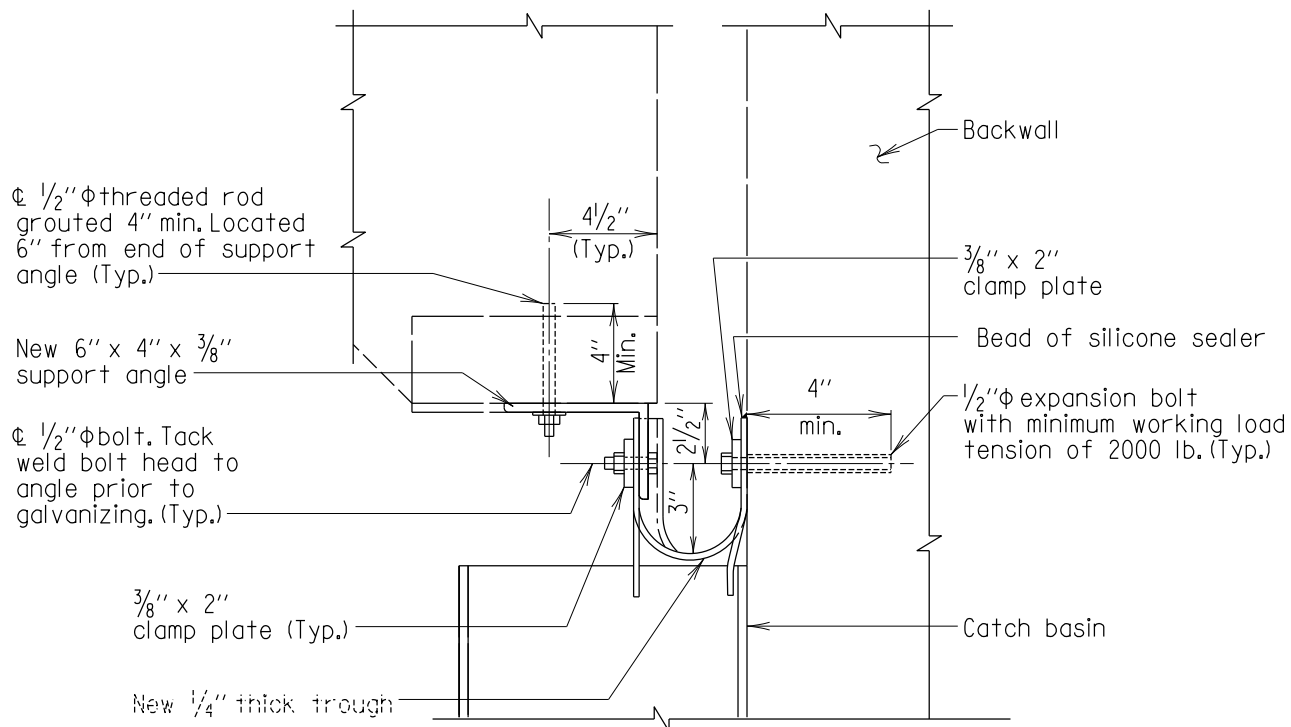
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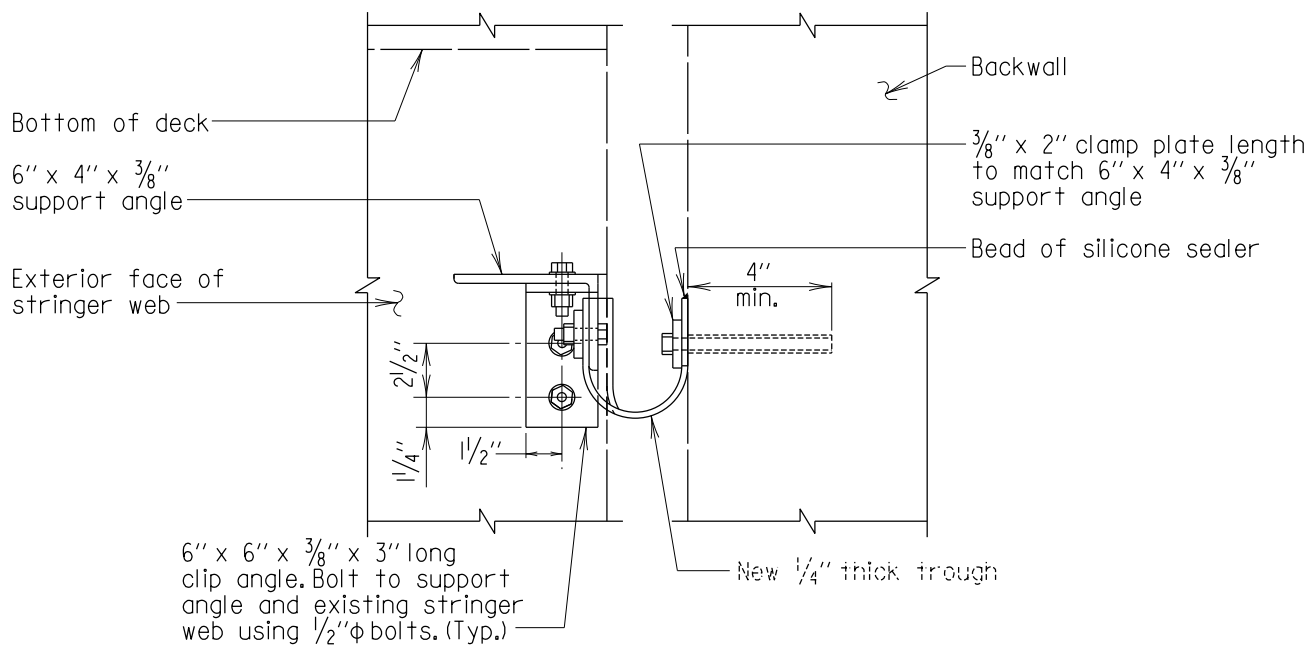
### DRAINAGE TROUGH DETAIL AT ABUTMENT FOR EXISTING STRUCTURE

STANDARD NO. BR-SR(0.06)-95-309

SHEET 3 OF 7



SECTION C-C  
Scale: 1 1/2" = 1'-0"



SECTION D-D  
Scale: 1 1/2" = 1'-0"

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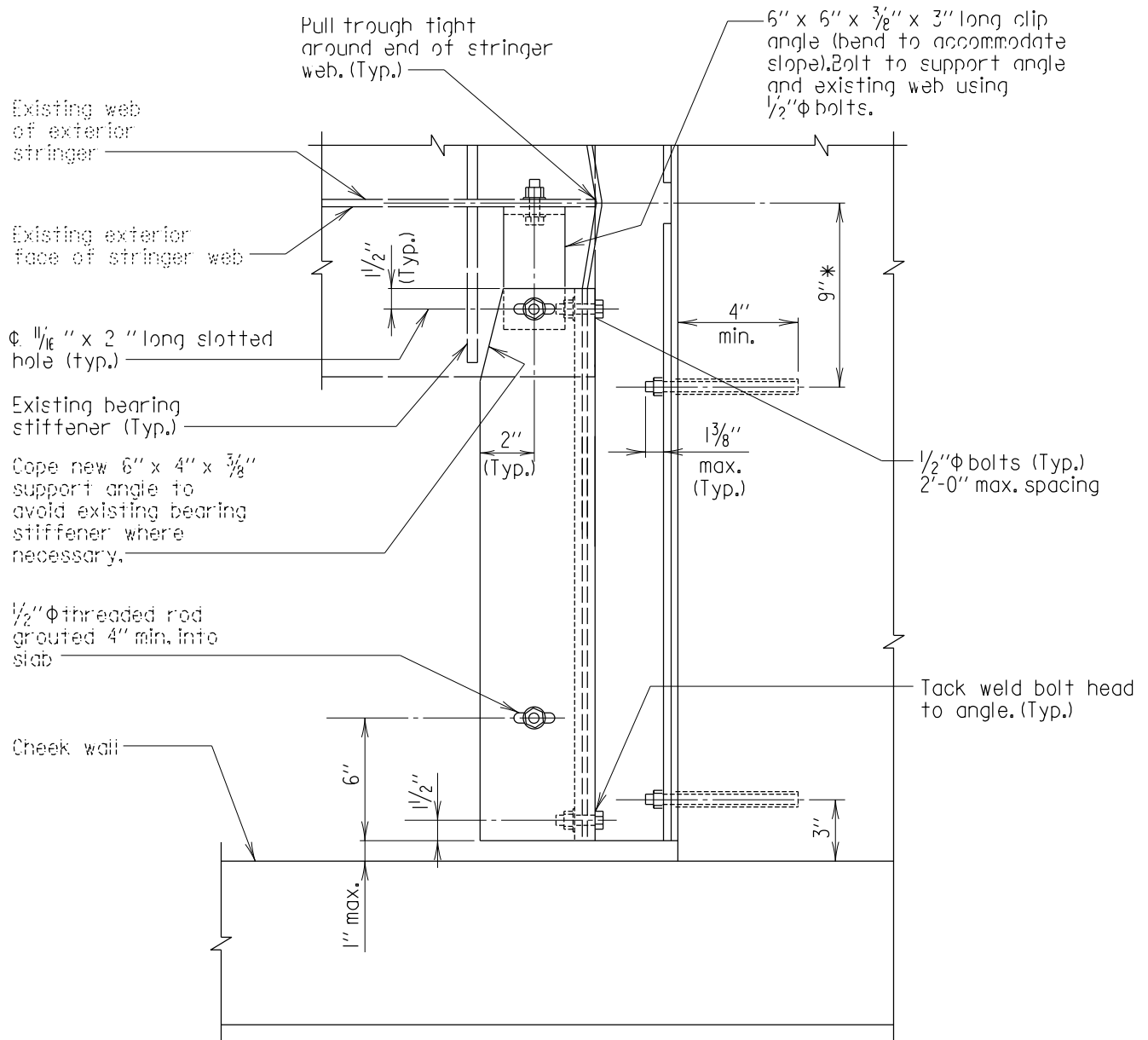
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DRAINAGE TROUGH DETAIL AT ABUTMENT  
FOR EXISTING STRUCTURE

STANDARD NO. BR-SR(0.06)-95-309

SHEET 4 OF 7

STRUCTURAL REPAIRS



SECTION E-E  
Scale:  $1\frac{1}{2}$ " = 1'-0"

\* On skewed bridges, these dimensions shall be adjusted as necessary to clear the stringer end bearing stiffeners.

Note:  
Existing interior seat angle not shown for clarity.

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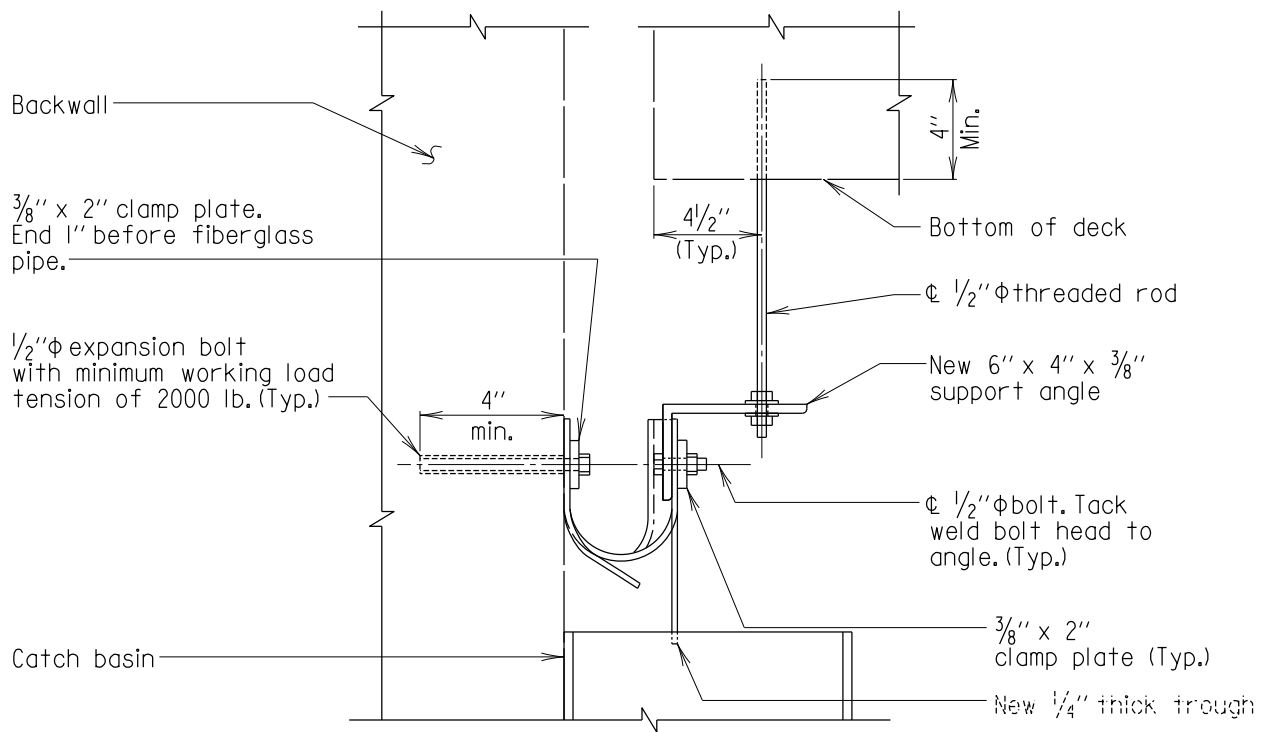
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DRAINAGE TROUGH DETAIL AT ABUTMENT  
FOR EXISTING STRUCTURE

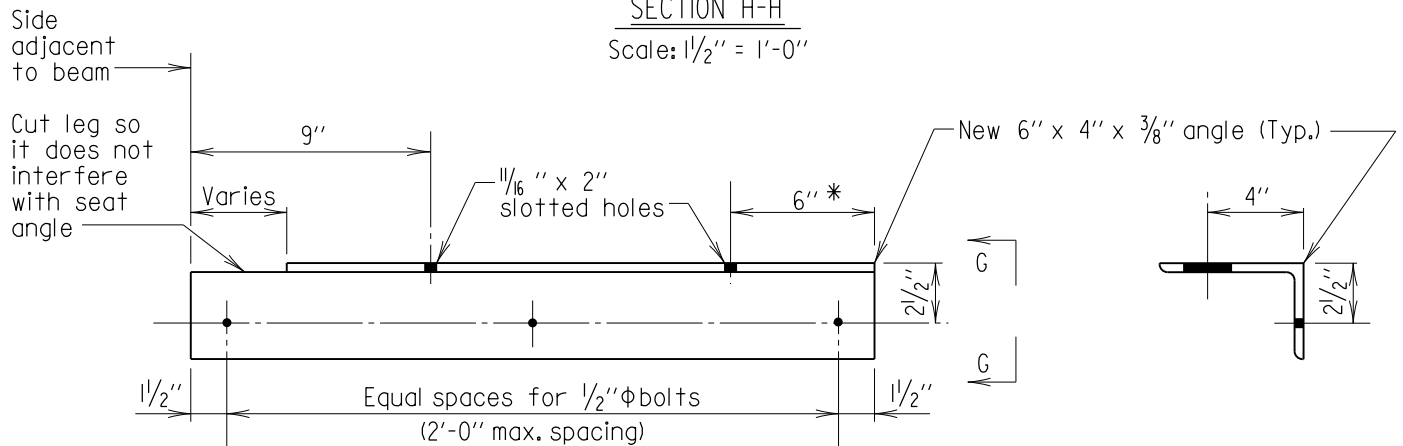
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SHEET 5 OF 7

STRUCTURAL REPAIRS



SECTION H-H  
Scale: 1 1/2" = 1'-0"



SUPPORT ANGLE AT DOWNSPOUT  
Scale: 1 1/2" = 1'-0"

VIEW G-G  
Scale: 1 1/2" = 1'-0"

\*When installed on the exterior,  
adjust to miss the elbow by 1 1/2".

Note:  
Length to be determined in the  
field.

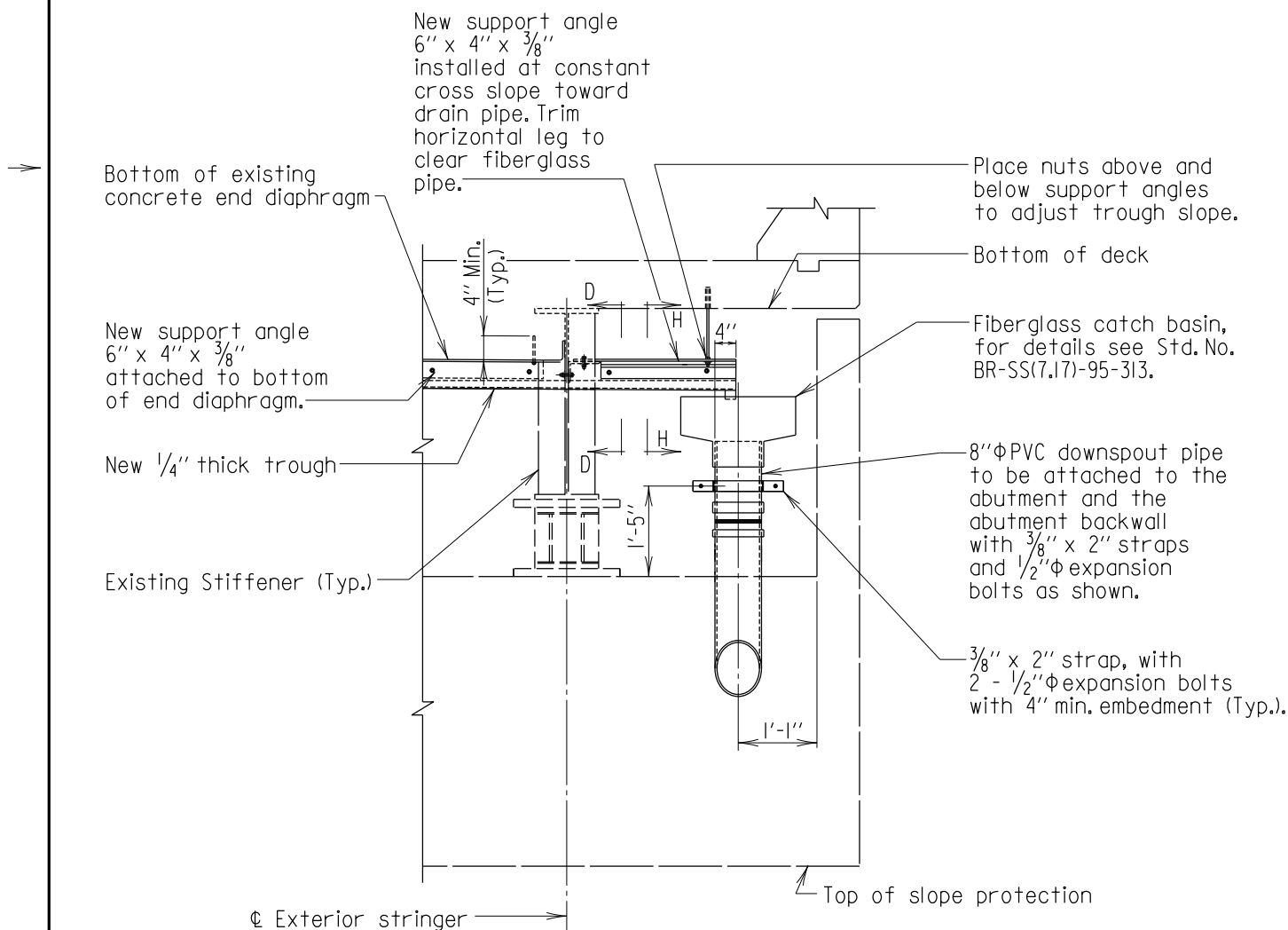
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DRAINAGE TROUGH DETAIL AT ABUTMENT  
FOR EXISTING STRUCTURE

STANDARD NO. BR-SR(0.06)-95-309

SHEET 6 OF 7



### DOWNSPOUT DETAIL AT END OF ABUTMENT

Scale:  $\frac{3}{8}$ " = 1'-0"

#### APPROVAL

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### DRAINAGE TROUGH DETAIL AT ABUTMENT FOR EXISTING STRUCTURE

STANDARD NO. BR-SR(0.06)-95-309

SHEET 7 OF 7

STRUCTURAL REPAIRS



## GENERAL NOTES

1. It is preferable to place a single layer of grout bags instead of stacking. Place filter fabric under all grout bags including a single layer of bags.
2. If bags are stacked, overlap the joints of the preceding layer.
3. If possible, bags should be placed so that the top of the bag is at or below the stream bottom. (When filling a scour hole, keep the top of the bag at or below the stream bottom).
4. If the stream bed consists of soils that allow for settlement of the grout bags, do not tie the bags together. If the stream bed consists of a hard stiff soil/clay or an erodable rock, which the grout bags will never be able to settle, tie the grout bags together so they do not get washed away.
5. Grout bags should be no larger than 3' wide, 4' long and 1' thick.
6. The bag placed directly in front of the nose of the pier should be the width of the exposed portion of the pier. Similarly, make sure no gaps form between the bags and the front face of the footing.
7. Do not overfill the bags or allow grout to be poured between the seams of two bags.

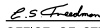
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STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES  GROUT BAG INSTALLATION GENERAL NOTES																	
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STRUCTURAL REPAIRS

## GENERAL NOTES

1. Depending on the depth of the undermining, place one grout bag or stack several layers of grout bags along the face of the abutment or pier in front of the undermined area. If bags are stacked, bags in successive rows and tiers shall be staggered.
2. Once the vent/fill pipes have been installed and the bags are filled, pump the grout into the undermined area until grout appears in the top of adjacent vent pipes. Cut or remove the vent/fill pipes flush with the top of the bags after the pumping operation is complete.
3. Adequate venting of the water to be displaced in the undermined area is important. The water must be able to escape when it is displaced by the grout pumped into the cavity. A 4' maximum spacing of the vent/fill pipes is recommended.
4. It is important to keep the nozzle buried in the grout during the pumping. This is to reduce the amount of mixing of the grout and the water to be displaced.
5. Debonding jackets should be placed around piles to prevent the grout from adhering to the piles if the exposed height is 3'-0" or greater. This is to prevent the additional weight of the grout from reducing the piles capacity.
6. If possible, clean out unstable material along the bottom of the undermined area prior to filling with grout.

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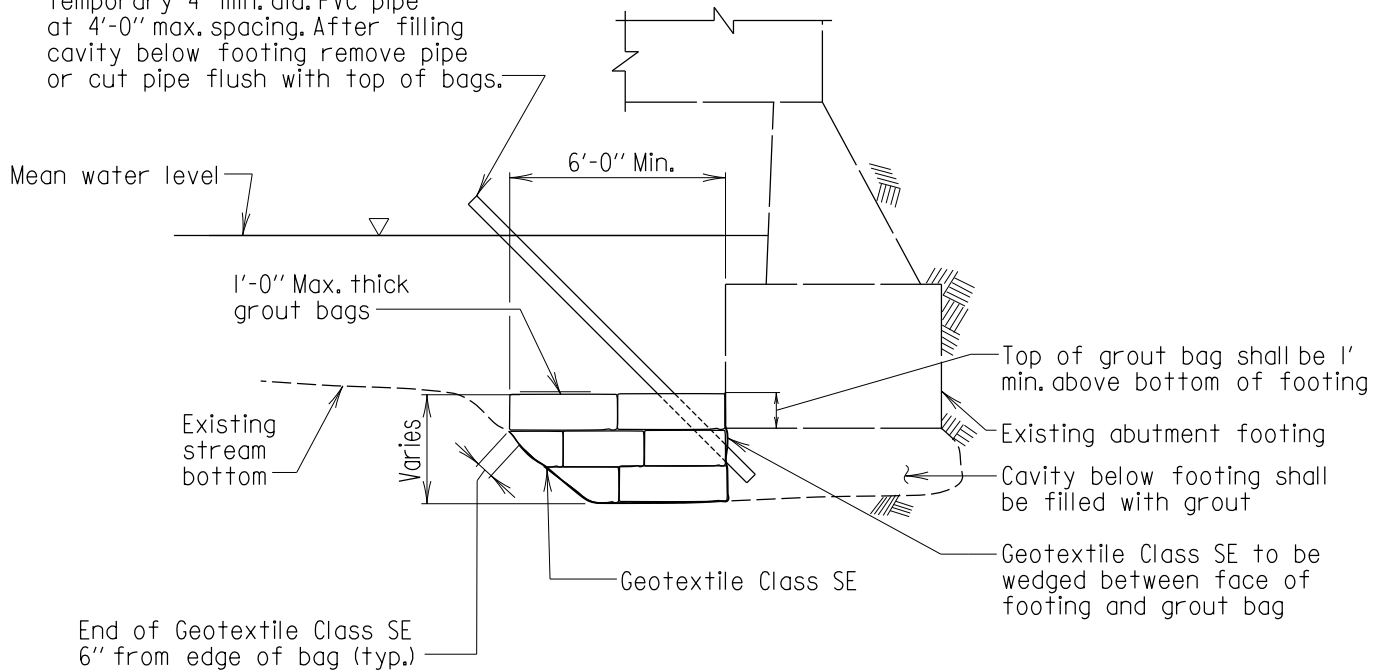
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GROUT BAG INSTALLATION FOR UNDERMINED  
AREAS AT PIERS AND ABUTMENTS  
GENERAL NOTES

STANDARD NO. BR-SR(0.08)-96-315

SHEET 1 OF 1

STRUCTURAL REPAIRS

Temporary 4" min. dia. PVC pipe  
at 4'-0" max. spacing. After filling  
cavity below footing remove pipe  
or cut pipe flush with top of bags.



### SECTION THRU ABUTMENT

Scale:  $\frac{3}{16}$ " = 1'-0"

#### Notes:

1. Stack bags as required. Joints between bags in successive rows and tiers shall be staggered.
2. Refer to General Plan for any excavation requirements.
3. Place top bag flush with face of footing.
4. If on piles, place debonding material around piles with greater than 3'-0" exposure.
5. All bags shall be 1 ft. max. thick, 3 ft. max. wide, and 4 ft. max. length.
6. Remove debris before installation of bags.

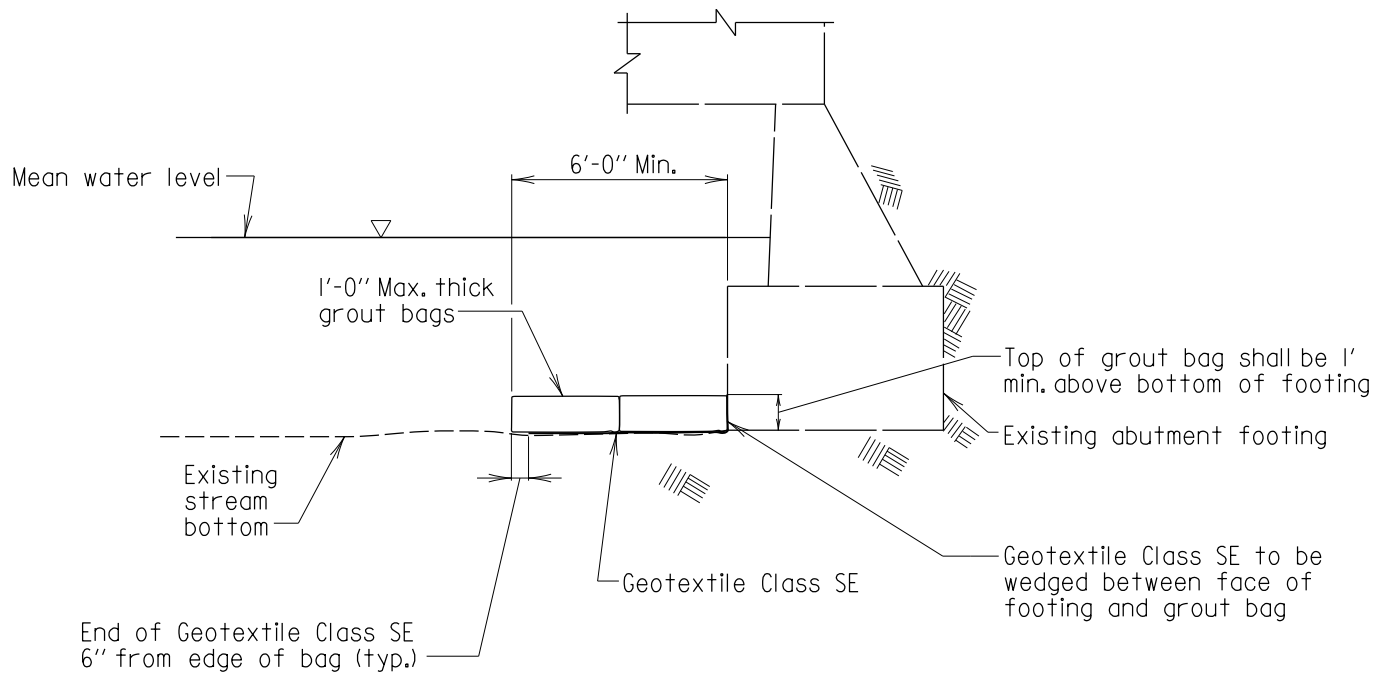
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GROUT BAG SECTION  
CASE WHERE SCOUR AND UNDERMINING HAS  
OCCURRED AT ABUTMENT

STANDARD NO. BR-SR(0.09)-96-316

SHEET 1 OF 7



SECTION THRU ABUTMENT  
Scale:  $\frac{3}{16}" = 1'-0"$

**Notes:**

1. Refer to General Plan for any excavation requirements.
2. Place bags flush with face of footing.
3. All bags shall be 1 ft. max. thick, 3 ft. max. wide, and 4 ft. max. length.

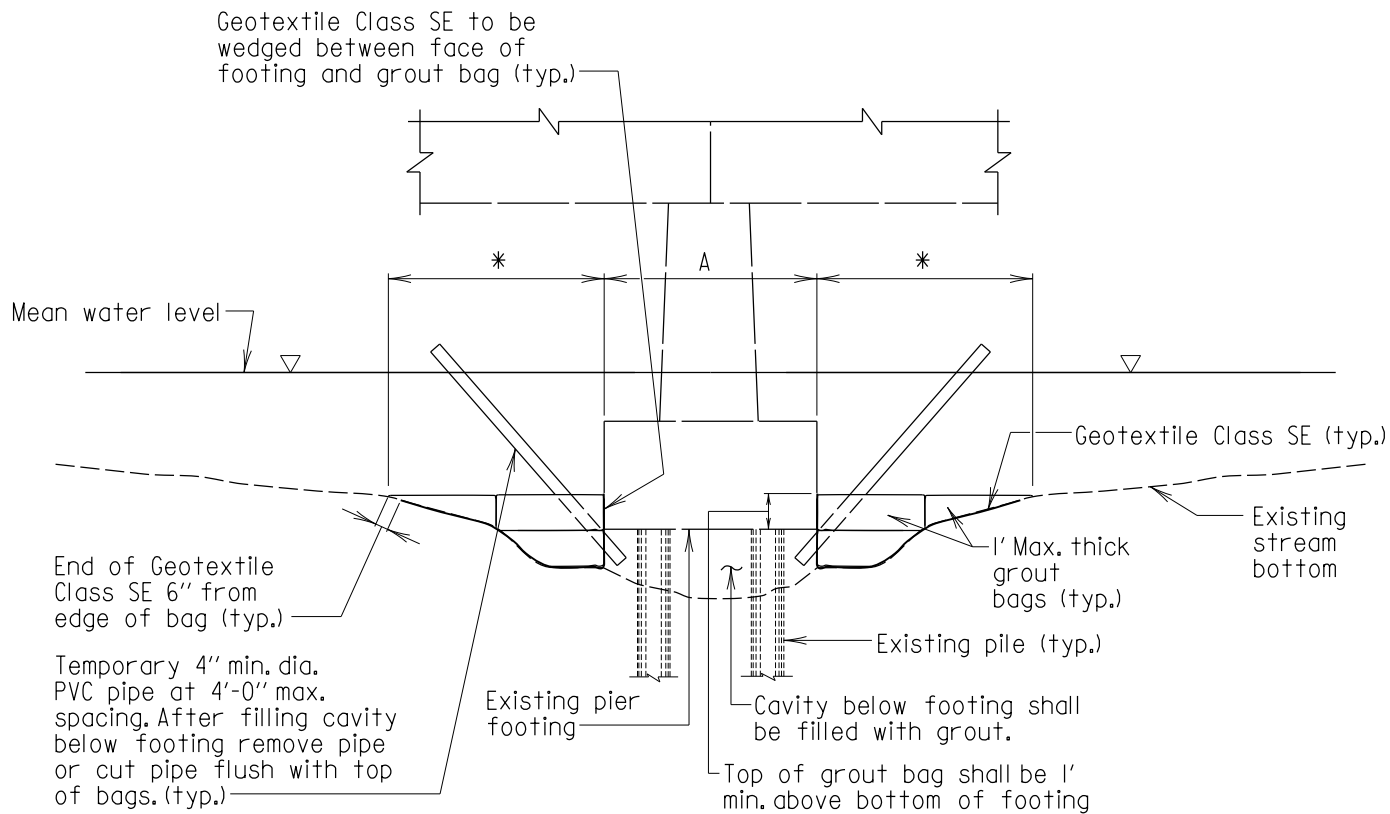
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**GROUT BAG SECTION  
CASE WHERE SCOUR POTENTIAL EXISTS  
AT ABUTMENT**

STANDARD NO. BR-SR(0.09)-96-316

SHEET 2 OF 7



### SECTION THRU PIER - ON PILES

Scale:  $\frac{3}{16}" = 1'-0"$

\* 2A or 6'-0", whichever is greater, with a maximum of 12'-0".

#### Notes:

1. Stack bags as required. Joints between bags in successive rows and tiers shall be staggered.
2. Refer to General Plan for any excavation requirements.
3. Place top bag flush with face of footing.
4. If on piles, place debonding material around piles with greater than 3'-0" exposure.
5. All bags shall be 1 ft. max. thick, 3 ft. max. wide, and 4 ft. max. length.
6. Remove debris before installation of bags.

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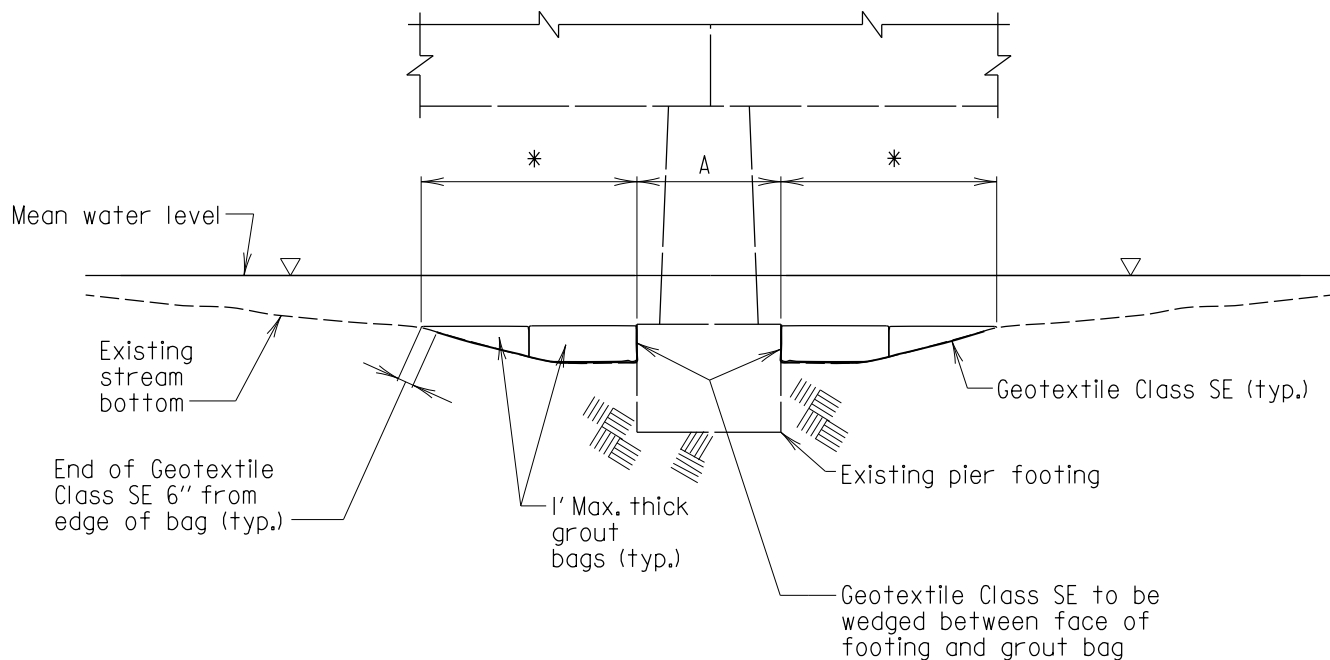
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GROUT BAG SECTION  
CASE WHERE SCOUR AND UNDERMINING  
HAS OCCURED

STANDARD NO. BR-SR(0.09)-96-316

SHEET 3 OF 7

STRUCTURAL REPAIRS



# SECTION THRU PIER

Scale:  $\frac{3}{16}'' = 1'-0''$

\* 2A or 6'-0'', whichever is greater, with a maximum of 12'-0''.

## Notes:

1. Refer to General Plan for any excavation requirements.
2. Place bags flush with face of footing.
3. All bags shall be 1 ft. max. thick, 3 ft. max. wide, and 4 ft. max. length.
4. Top of grout bags shall be 1 ft. min. above bottom of footing.
5. Refer to sheet 5 of 7 for plan view of grout bag installation at pier.

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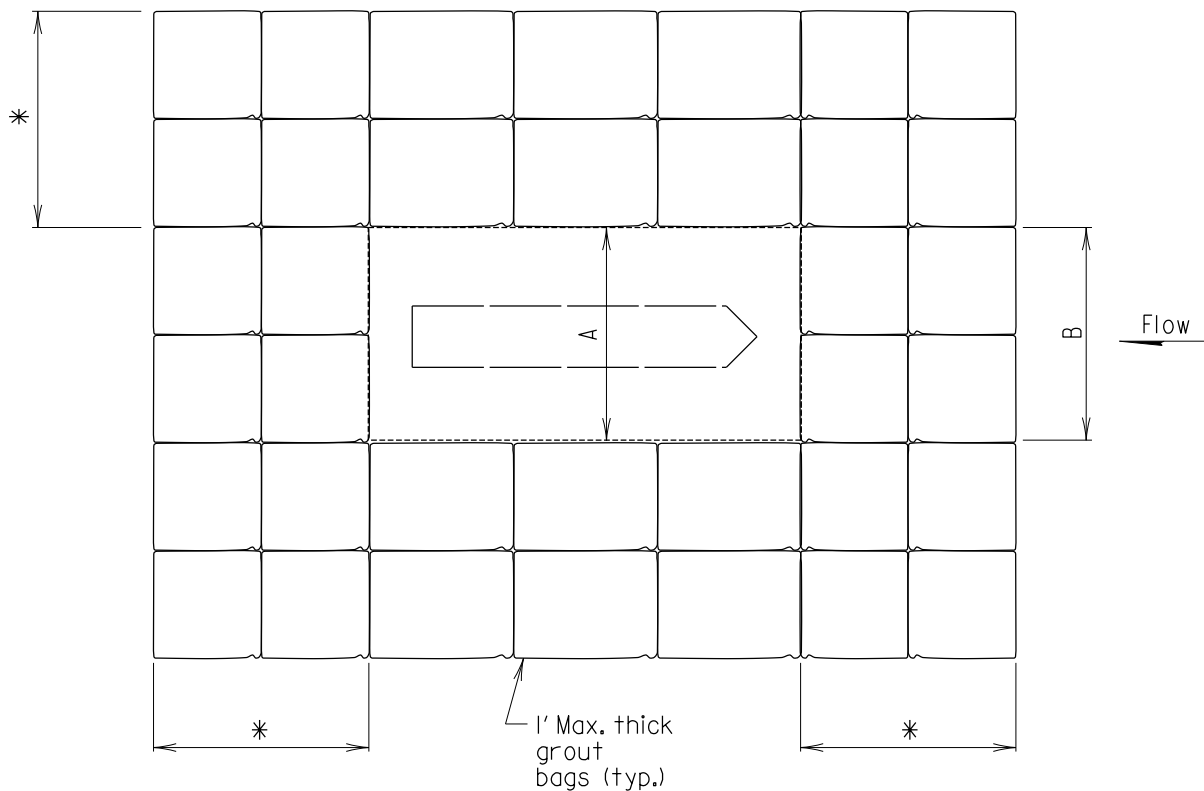
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## GROUT BAG SECTION CASE WHERE SCOUR POTENTIAL EXISTS AT PIER

STANDARD NO. BR-SR(0.09)-96-316

SHEET 4 OF 7



PLAN OF PIER  
Scale:  $\frac{3}{16}$ " = 1'-0"

- \* 2A or 6'-0", whichever is greater, with a maximum of 12'-0".
- A= Width of pier footing.
- B= Length of grout bags in front and behind pier to match pier footing width.

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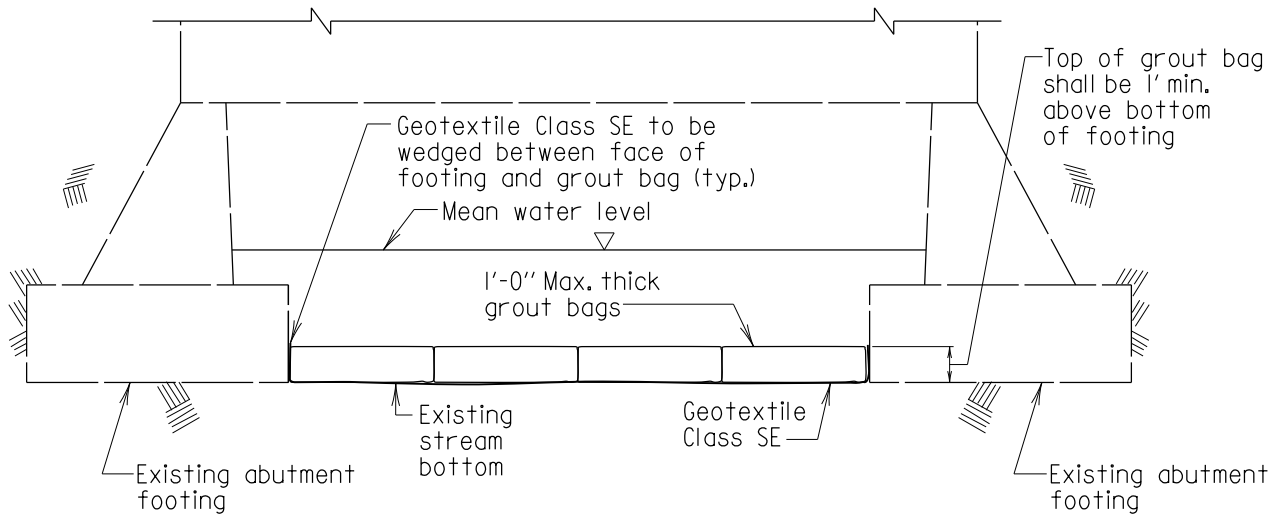
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PLAN VIEW OF GROUT BAG  
INSTALLATION AT PIER

STANDARD NO. BR-SR(0.09)-96-316

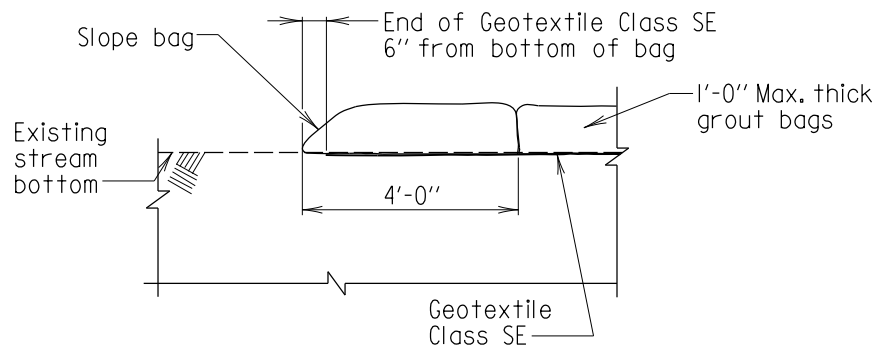
SHEET 5 OF 7

Note:  
Grout bag entire stream channel  
for clear spans measuring perpendicular  
between footings of 16 ft. and less.



### SECTION THRU ABUTMENTS AND CHANNEL

Scale:  $\frac{3}{16}" = 1'-0"$



Note:  
For location of Section A-A  
see sheet 7 of 7.

### SECTION A-A

Scale:  $\frac{1}{4}" = 1'-0"$

#### Notes:

1. Lay bags on top of existing stream bottom.
2. Bags shall be buried at the inlet and outlet end of the structure.
3. Refer to General Plan for any excavation requirements.
4. Place bag flush with face of footing.

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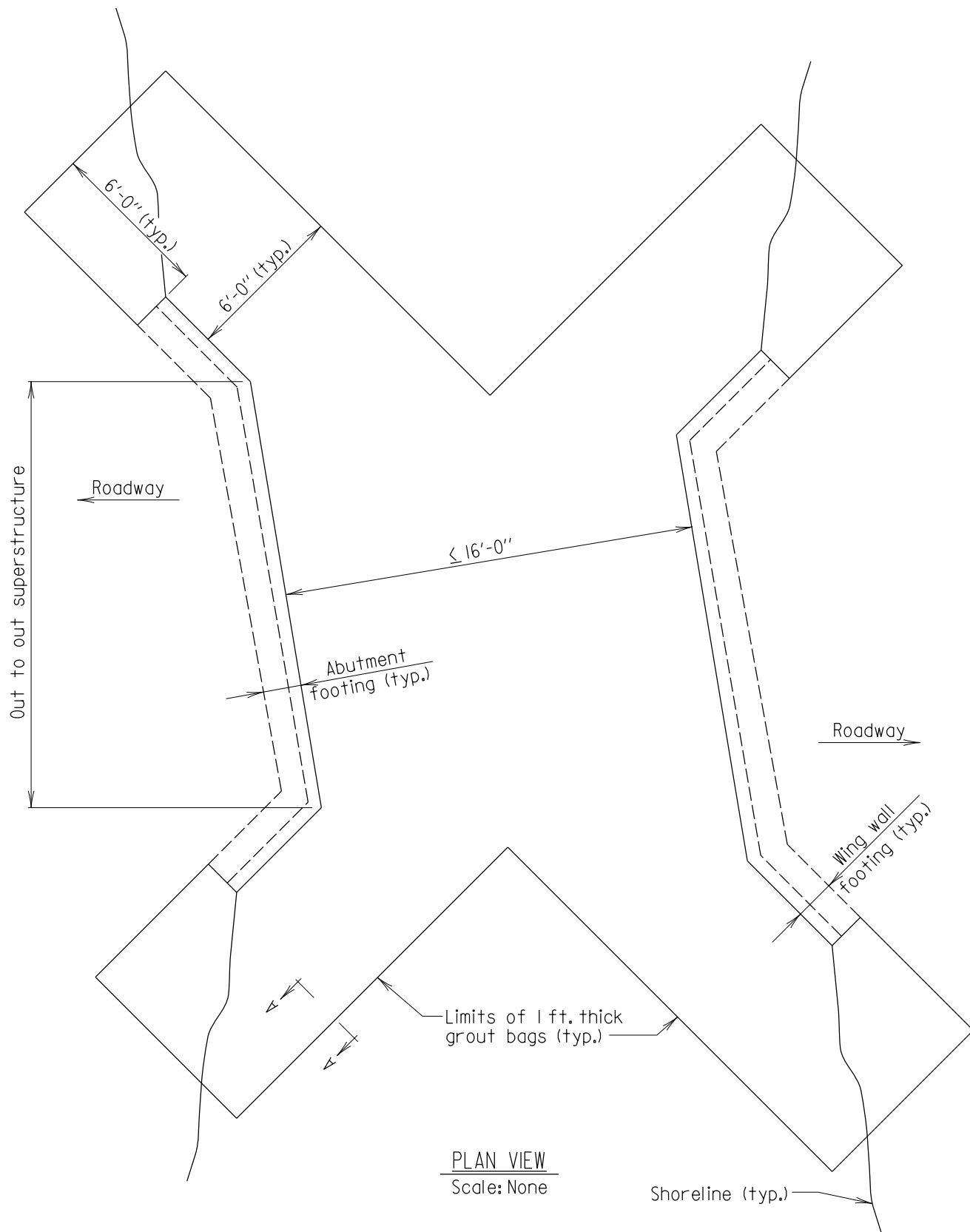
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SECTION VIEW OF GROUT BAGS  
CASE WHERE SCOUR POTENTIAL EXISTS  
FOR FULL CHANNEL WIDTH

STANDARD NO. BR-SR(0.09)-96-316

SHEET 6 OF 7

STRUCTURAL REPAIRS





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PLAN VIEW OF GROUT BAGS  
CASE WHERE SCOUR POTENTIAL EXISTS  
FOR FULL CHANNEL WIDTH

STANDARD NO. BR-SR(0.09)-96-316

SHEET 7 OF 7

STRUCTURAL REPAIRS

## GENERAL NOTES

- Wire Rope:** Wire rope shall meet the requirements of Federal Specification RR-W-410D, Latest Edition, Type 304 Stainless Steel 7 x 19 IWRC aircraft cable, Extra improved plow steel. The cable shall be 1/4" diameter having a minimum breaking load of 6400 lbs and a working and a working load limit of 1400 lbs.
- Clips:** Wire rope clips shall be stainless steel and meet Federal Specifications FF-C-450D, Type I, Class I. A minimum of 3 clips shall be installed at each loop fitting. Nuts for clips shall be tightened to a torque of 15 ft-lbs.
- Thimbles:** Wire rope thimbles shall be 1/4" heavy stainless steel and meet Federal Specification FF-T-276b, Type III.
- Turnbuckle:** Turnbuckles shall be 1/2" diameter with a 6" take up and jaw end fittings at both ends that meet Federal Specification FF-T-791B, Type I, Form I, Class 7. Turnbuckles shall be forged stainless steel, quenched and tempered. Turnbuckles shall have a minimum breaking load of 11,000 lbs and a working load limit of 2,200 lbs.
- Steel Pipe:** Stainless steel pipes shall be nominal 3/4" diameter standard weight pipe meeting A 53, Grade B furnished to the dimensions shown on the Plans. The outside edges of the pipe shall be deburred to prevent damage to the wire rope.
- Epoxy Adhesive:** Refer to SHA Specification 921.04.

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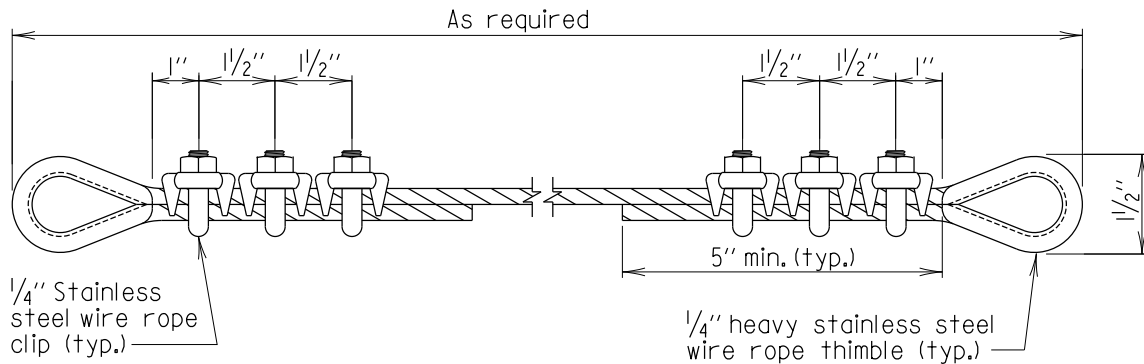
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### NOISE WALL PANEL RESTRAINT DETAIL GENERAL NOTES

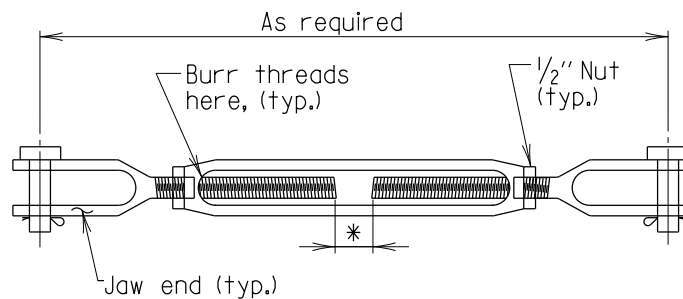
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SHEET 1 OF 2



### DETAIL OF 1/4" STAINLESS STEEL WIRE ROPE

Scale: None



\* Allow a 1/2" minimum gap after wire rope is taken up to snug tightness.

### DETAIL OF 1/2" DIAMETER STAINLESS STEEL TURNBUCKLE

Scale: None

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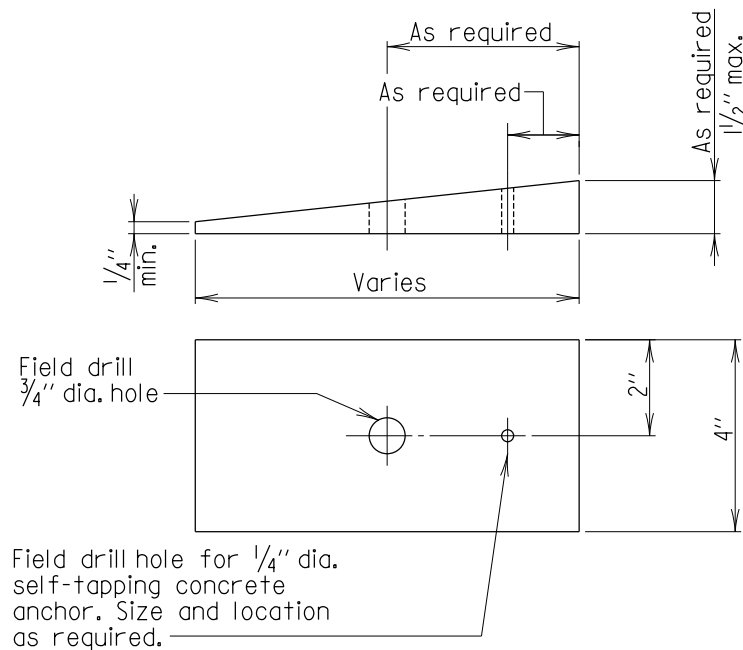
NOISE WALL PANEL RESTRAINT DETAIL  
HARDWARE

STANDARD NO. BR-SR(0,10)-07-378

SHEET 2 OF 2

## CONSTRUCTION SEQUENCE

1. Core a 1/4" dia. hole through the existing panel at locations as shown on panel elevation view of appropriate standard sheet.
2. Coat the outside face of the stainless steel pipe and the inside of the 1/4" dia. hole with epoxy adhesive as specified in the General Notes. Allow epoxy to set before threading or tensioning the wire rope.
3. Loop wire rope through stainless steel pipe and fasten turnbuckle as shown on Section A-A of appropriate standard sheet. The turnbuckle shall be taken up to snug tightness and tightened down with the jam nut. Burr turnbuckle threads after tightening is complete.
4. Coat those areas of the neoprene wedges in contact with concrete with lubricant adhesive conforming to 911.04.03 before inserting.



## NEOPRENE WEDGE DETAIL

Scale: 1/4" = 1"

Note:  
For neoprene specifications refer to M220-67. Color to match existing noise wall.

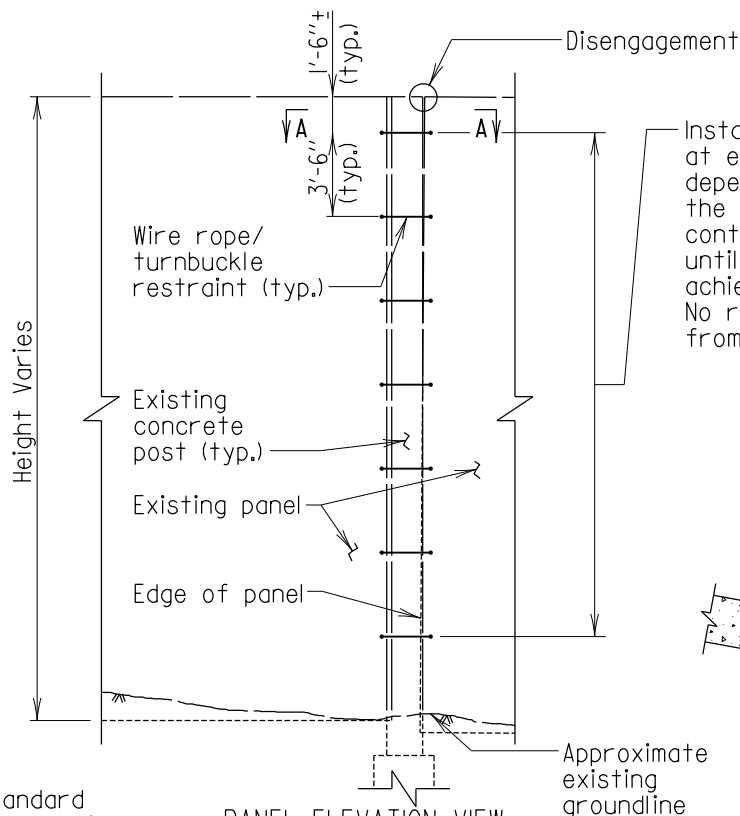
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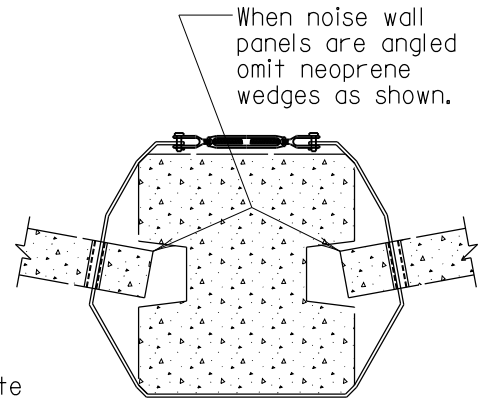
## NOISE WALL PANEL RESTRAINT DETAIL WEDGE

STANDARD NO. BR-SR(0,11)-07-379

SHEET 1 OF 1



Install cable restraints, starting at either the top or bottom, depending on the location of the disengagement, and continuing downward/upward until the edge of the panel has achieved 1" of engagement. No restraints are required from that point.

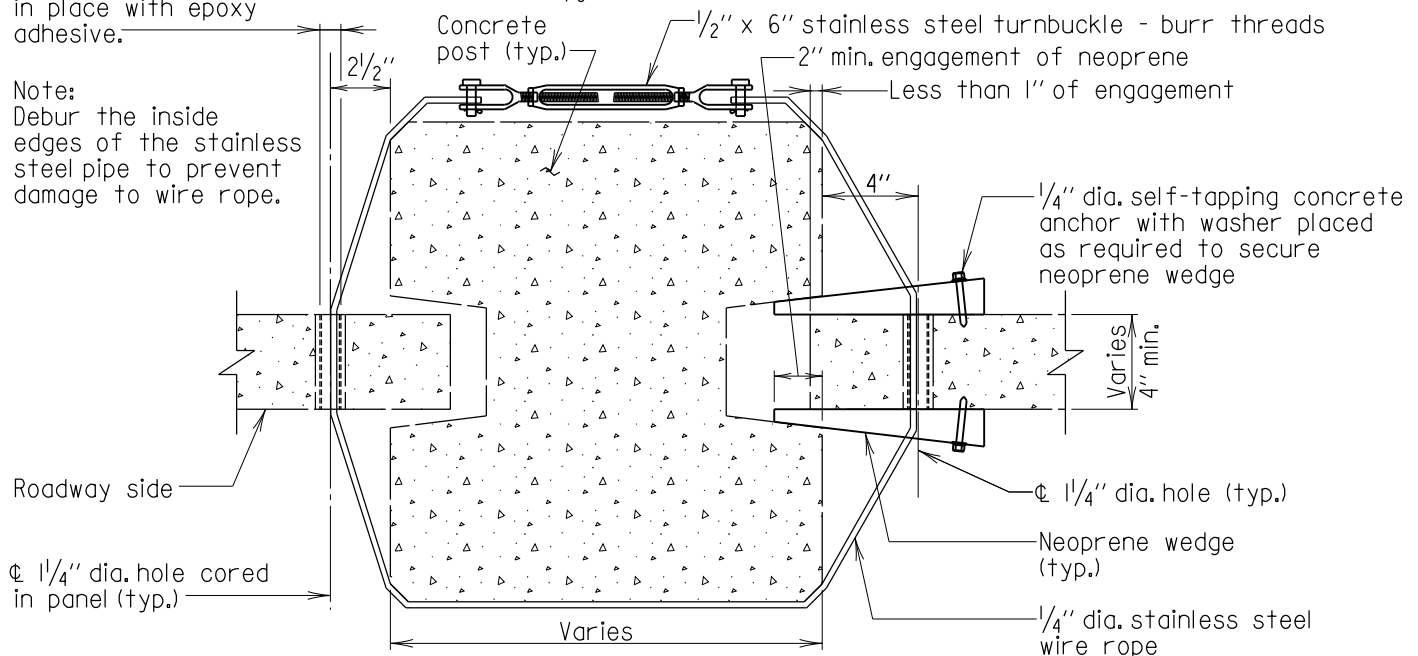


ALTERNATE SECTION A-A

Scale:  $\frac{3}{4}'' = 1'-0''$

Nominal  $\frac{3}{4}''$  dia. standard weight stainless steel pipe (typ.) in a  $1\frac{1}{4}''$  hole; held in place with epoxy adhesive.

Note:  
Debur the inside edges of the stainless steel pipe to prevent damage to wire rope.



SECTION A-A

Scale:  $1\frac{1}{2}'' = 1'-0''$

Note:  
On multi-sectional panels install a minimum of two panel restraints per section. Restraints shall be spaced a minimum of 1'-6" from the horizontal panel joint.

Noise Wall ID No:

Post No(s):

Direction for Counting Posts:

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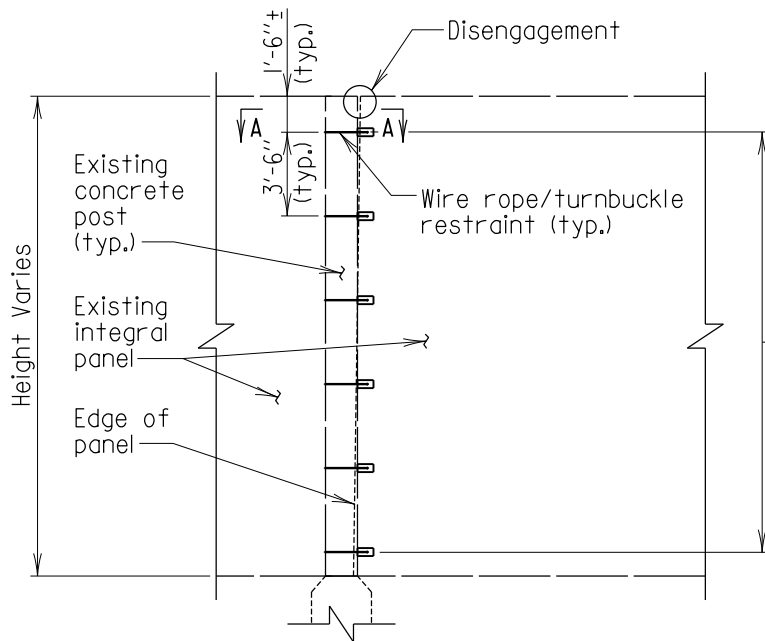
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NOISE WALL PANEL RESTRAINT DETAILS  
STAND ALONE CONCRETE POSTS

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STANDARD NO. BR-SR(0,12)-07-380

SHEET 1 OF 1



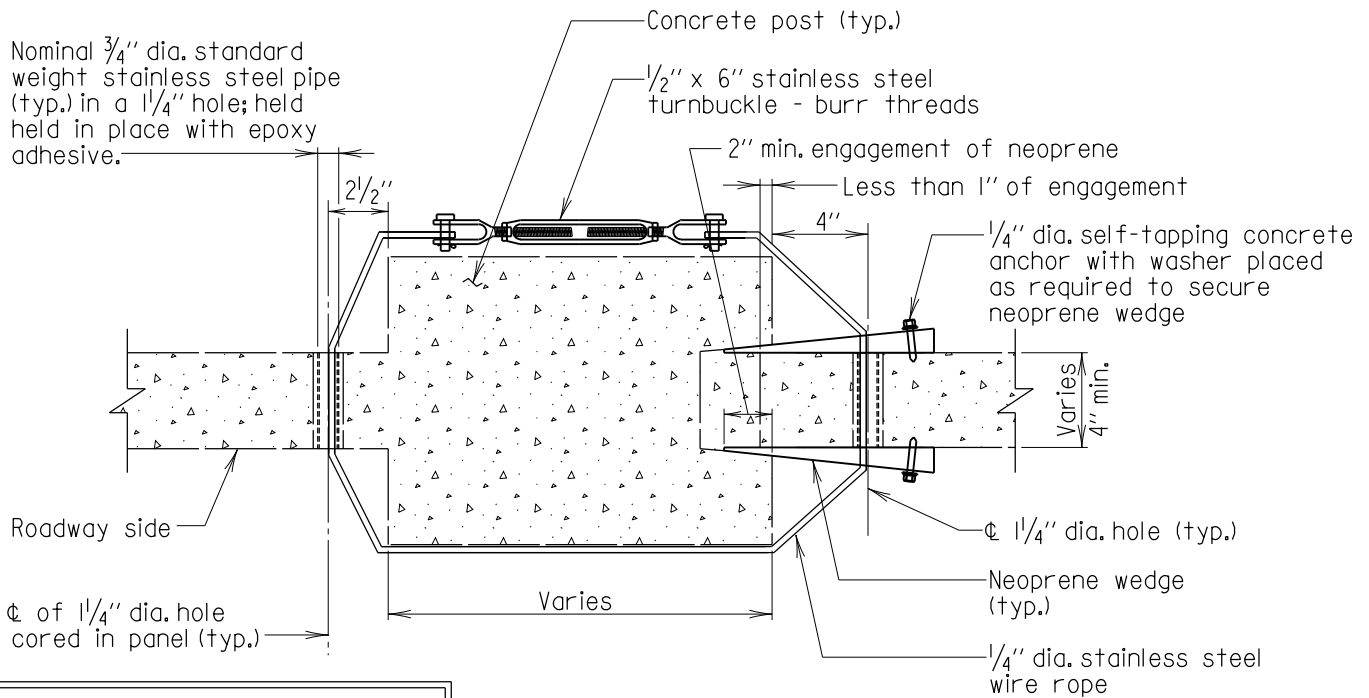
Install cable restraints, starting at either the top or bottom, depending on the location of the disengagement, and continuing downward/upward until the edge of the panel has achieved 1" of engagement. No restraints are required from that point.

**PANEL ELEVATION VIEW**

Scale:  $\frac{1}{8}" = 1'-0"$

Note:  
Debur the inside edges of the stainless steel pipe to prevent damage to wire rope.

Nominal  $\frac{3}{4}"$  dia. standard weight stainless steel pipe (typ.) in a  $1\frac{1}{4}"$  hole; held in place with epoxy adhesive.



**SECTION A-A**

Scale:  $\frac{1}{2}" = 1'-0"$

Note:  
On multi-sectional panels install a minimum of two panel restraints per section. Restraints shall be spaced a minimum of 1'-6" from the horizontal panel joint.

Noise Wall ID No:

Post No(s):

Direction for Counting Posts:

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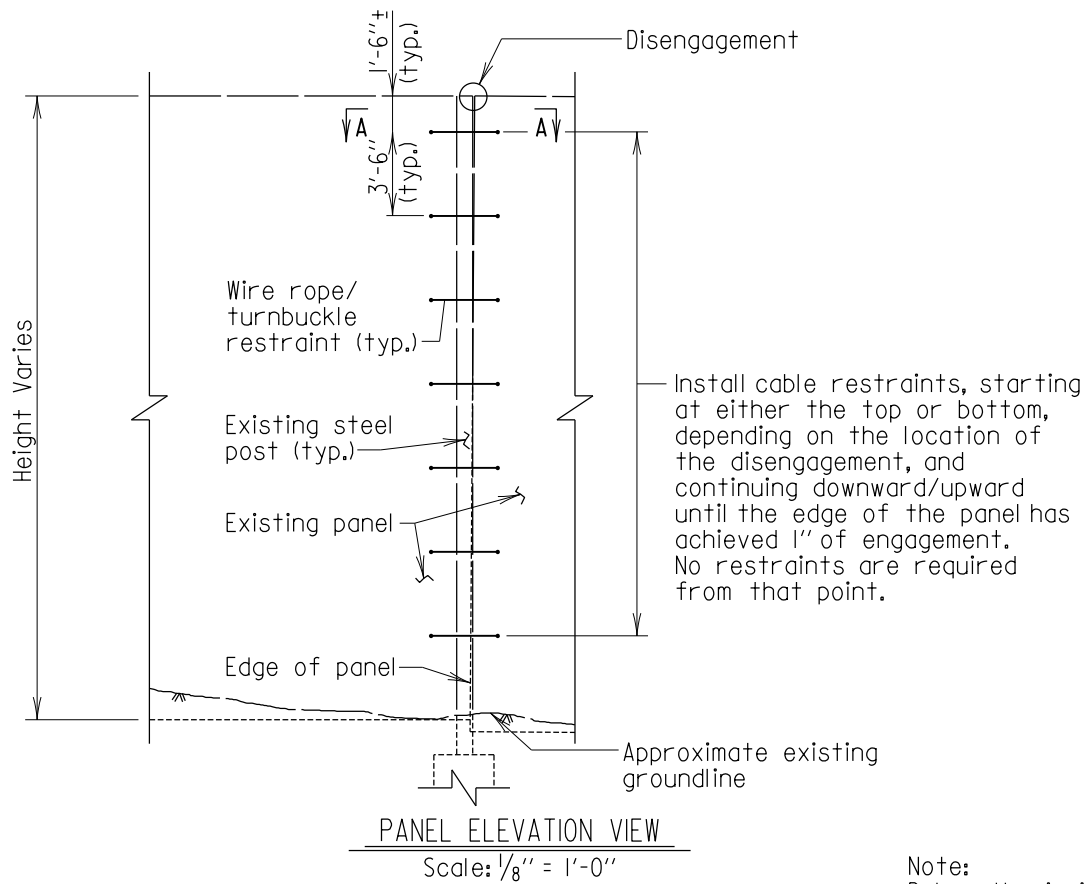
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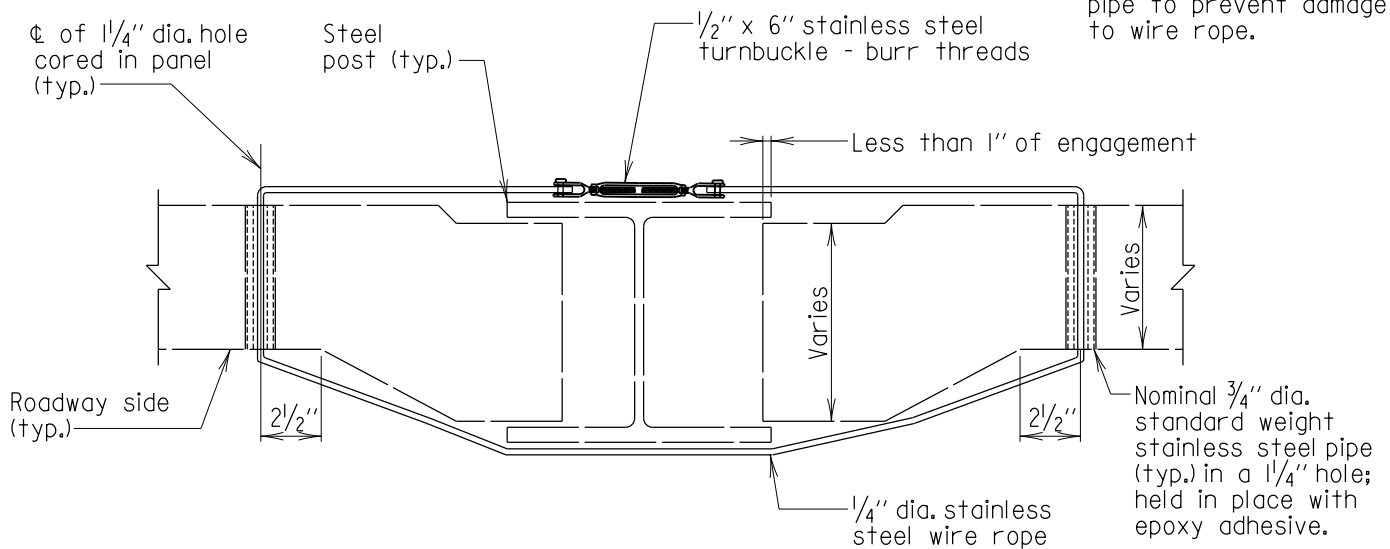
NOISE WALL PANEL RESTRAINT DETAILS  
CONCRETE NOISE WALL WITH INTEGRAL POSTS

STANDARD NO. BR-SR(0,13)-07-381

SHEET 1 OF 1



Note:  
Debur the inside edges of the stainless steel pipe to prevent damage to wire rope.



Note:  
On multi-sectional panels install a minimum of two panel restraints per section. Restraints shall be spaced a minimum of 1'-6" from the horizontal panel joint.

Noise Wall ID No:

Post No(s):

Direction for Counting Posts:

#### SECTION A-A

Scale: 1 1/2" = 1'-0"

#### APPROVAL

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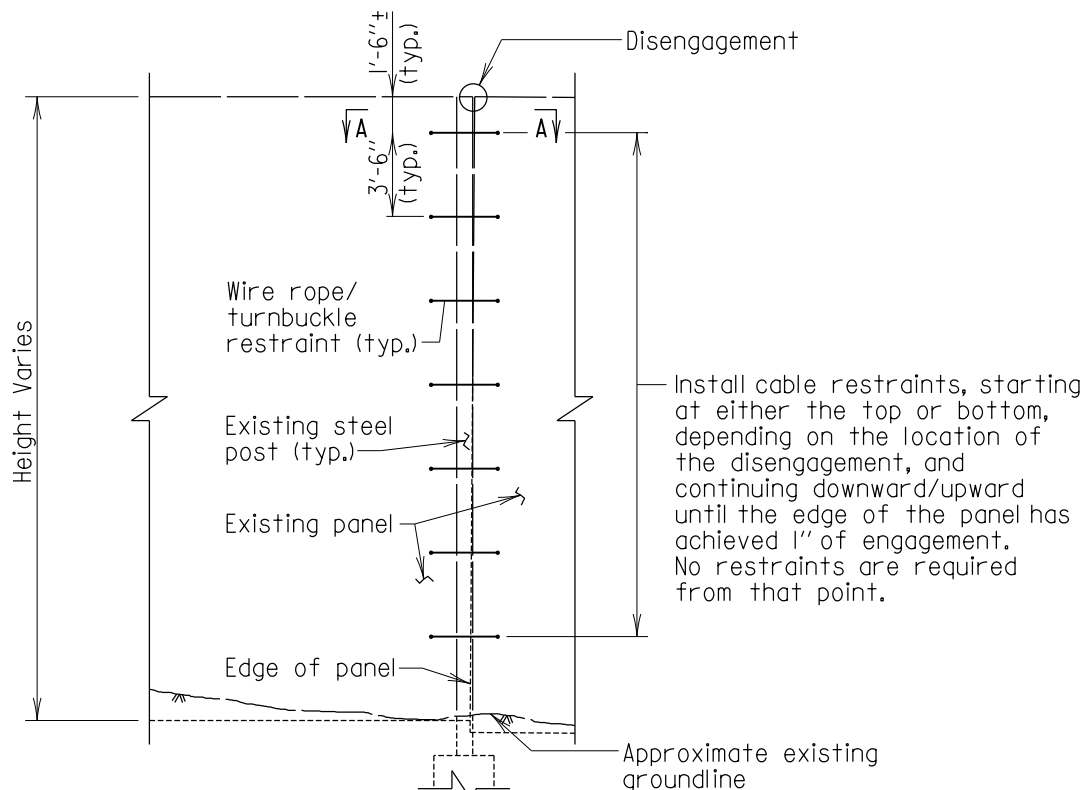
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#### NOISE WALL PANEL RESTRAINT DETAILS STEEL POSTS

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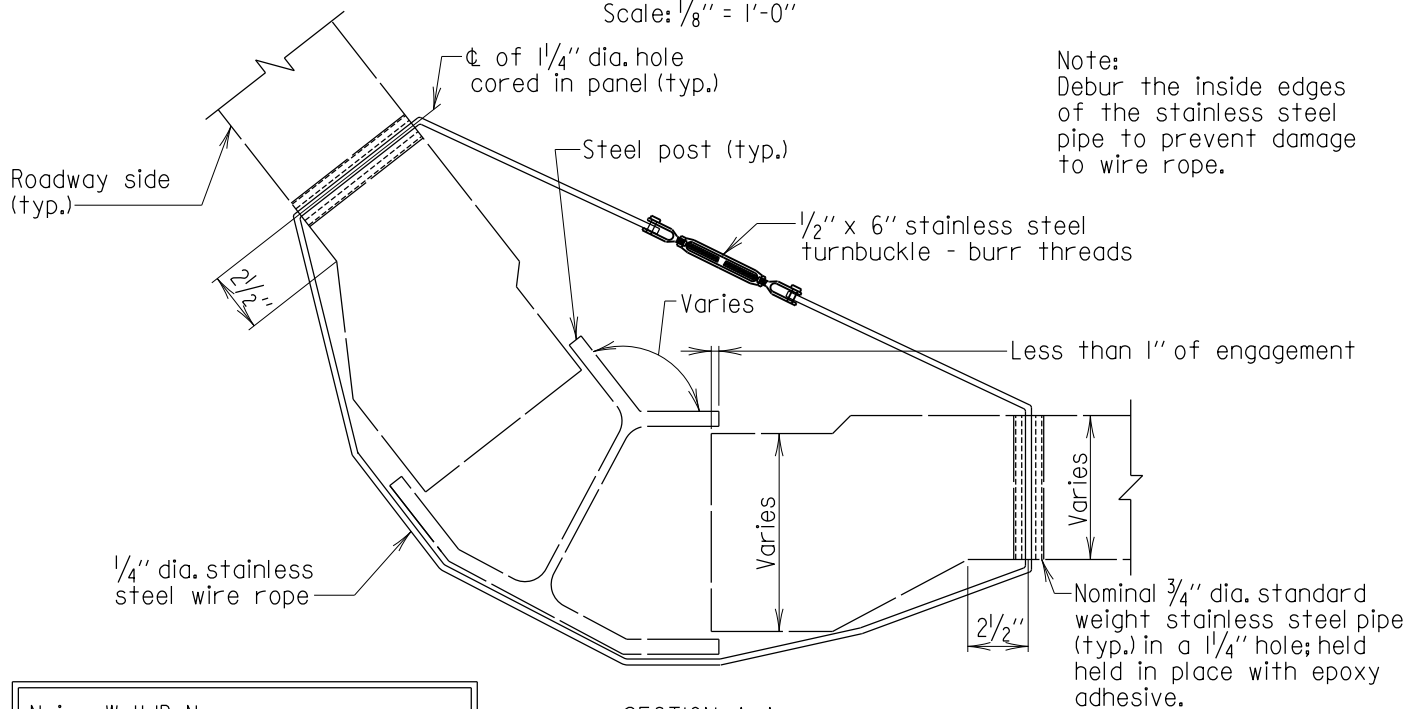
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SHEET 1 OF 1



**PANEL ELEVATION VIEW**

Scale:  $\frac{1}{8}" = 1'-0"$



**SECTION A-A**

Scale:  $\frac{1}{2}" = 1'-0"$

Noise Wall ID No:

Post No(s):

Direction for Counting Posts:

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**NOISE WALL PANEL RESTRAINT DETAILS  
SKEWED PANEL CONNECTION - SPECIAL STEEL POSTS**

STANDARD NO. BR-SR(0,15)-07-383

SHEET 1 OF 1

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